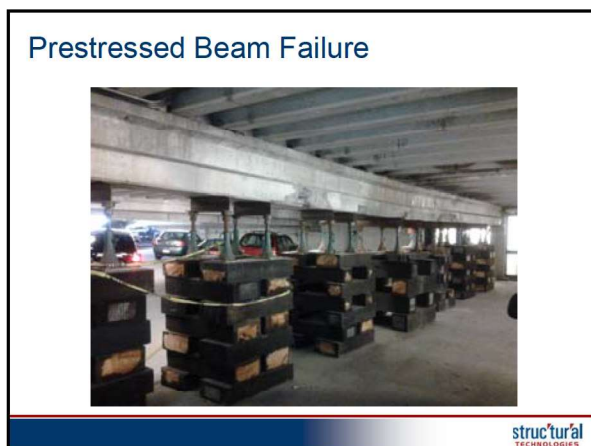


Strengthening and Repair of Concrete Structures

Toronto, February 21, 2020







Strengthening and Repair of Concrete Structures

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Environmental



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Environmental



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Renovation (Adaptive Reuse)



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Strengthening and Repair of Concrete Structures

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Design, Construction...Error



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Seismic Upgrade



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Failed Double – T-Stem



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Strengthening and Repair of Concrete Structures

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Design, Construction...Error



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Strengthening - Conventional Options



- Section enlargement



- Span shortening



- External Post-Tensioning



- Steel Plate Bonding

structural
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Flexural Strengthening

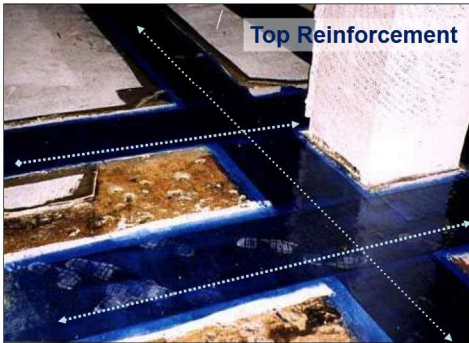


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Flexural Strengthening



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Slab Openings – Flexural Strengthening

New exhaust
duct opening



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Beam Shear Upgrade



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Column Strengthening



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Lateral Resisting System Upgrade

- New 23 Story Hi-rise Building



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Underground Pipelines



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Strengthening and Repair of Concrete Structures

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Surface Prep – CRITICAL!

Methods:

- Abrasive Blasting
- Dustless Grinding & Vacuum
- Water Blast & Dry



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Surface Prep – CRITICAL!

Desired Surface Profile: CSP-3



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FRP Installation



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Completed Bridge Pier - FRP Strengthening



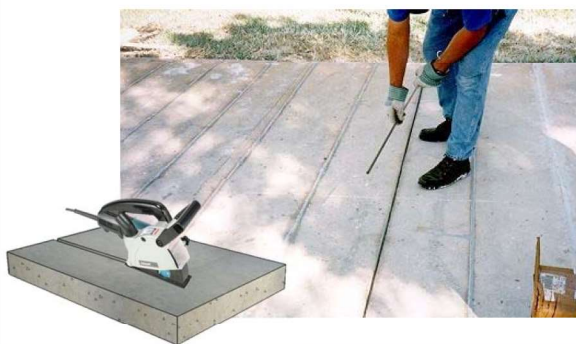
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Near-Surface Mounted (NSM)
FRP Bars



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Near Surface Mounted (NSM) FRP



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FRP Rods

Carbon:

#3 (T10)

#4 (T12)

Glass:

All standard sizes



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FRP Rods

Typical groove size
 $\frac{3}{4}$ " (18mm) X $\frac{3}{4}$ " (18mm)



Confirm rebar depth > $\frac{3}{4}$ "
(18mm) with GPR or other!



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FRP Rods

- Fill and strike off with margin trowel and pull tape



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FRP Rods – Finished Surface





V-Wrap FRP Systems



V-Wrap™ FRP Systems



Carbon Fabric



Glass Fabric



Carbon Rods



Epoxy



Carbon Plate



Anchors



Strengthening and Repair of Concrete Structures

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V-Wrap™ FRP Systems

Code Approved Products (ICC-ESR 3606)

- Epoxies:
 - Low Viscosity: V-Wrap 770
 - Putty: V-Wrap PF
- Carbon Fabric
 - High Strength: V-Wrap C100 / C200H / C400H
 - High Modulus: V-Wrap C200HM / C400HM
- Glass Fabric:
 - V-Wrap EG50 / EG50B
- Bi-directional:
 - Carbon: V-Wrap C220B
 - Glass: EG50B



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FRP Certification

- Building Officials (ICC-ES) Approved
- NSF Approved (potable water safe)
- UL Approved (fire rated protection)

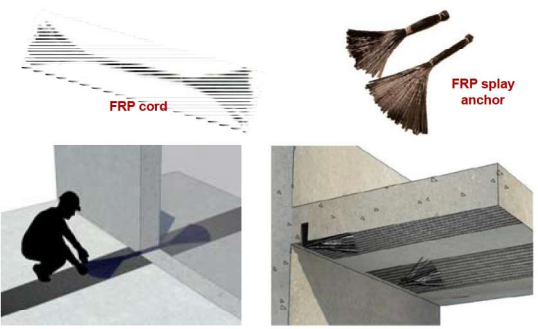
➤ Manufacturing Facilities Audits

➤ Periodic Testing



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FRP Anchors



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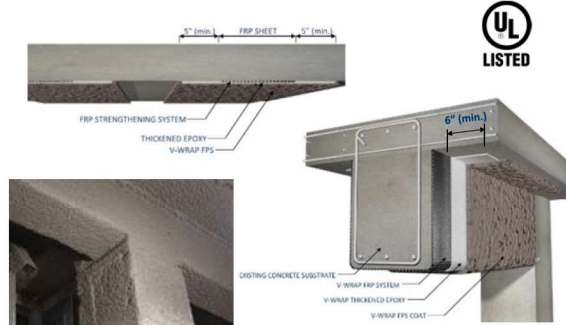
V-WRAP FPS Fire Protection System

- Single-component spray applied material
- Cementitious-based, non-combustible, and non-flammable
- Code Council approved (ESR-3606)
- **Advantages:**
 - UL listed product
 - Up to 4-hour rating
 - Very high bond strength
 - Fast and efficient application
 - Surface Flame Spread: 0
 - Smoke Developed: 0



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V-WRAP FPS TYPICAL APPLICATIONS



UL
LISTED

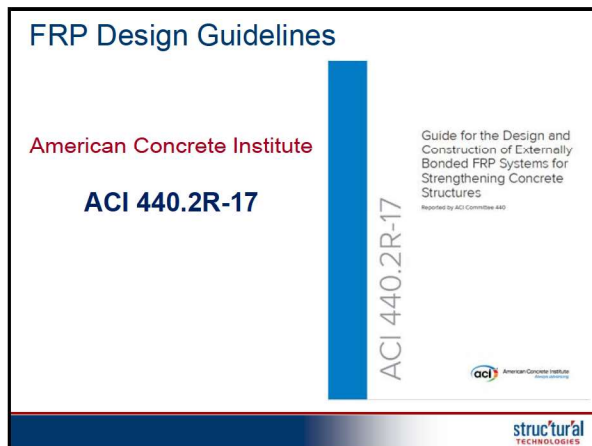
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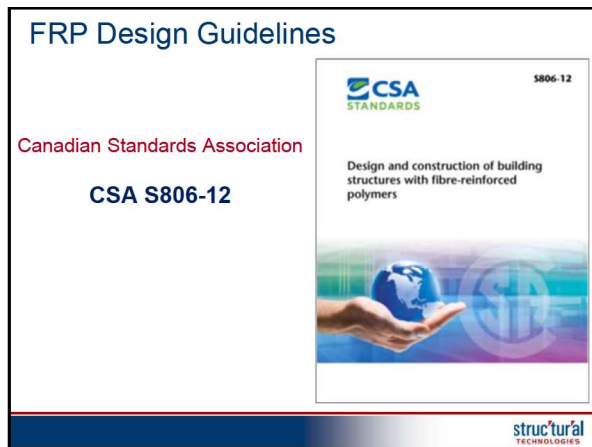
FRP Design Principles & Engineering Considerations

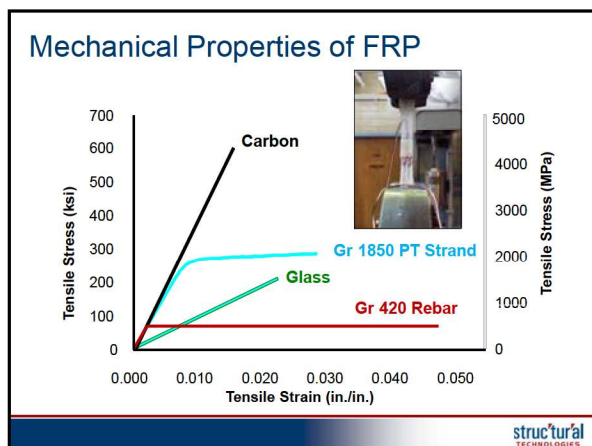
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Strengthening and Repair of Concrete Structures

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Strengthening and Repair of Concrete Structures

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FRP Test Methods

Properties	Test Method	Number of Specimens
Tensile Strength	ASTM D3039	20
Elongation	ASTM D3039	20
Tensile Modulus	ASTM D3039	20
Coefficient of Thermal Expansion (CTE)	ASTM D696 or E1142	5
Creep	ASTM D2990	5
Glass Transition Temperature	ASTM D4065	5

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ACI 440.2R Environmental Reduction Factor

Exposure condition	Fiber and resin type	C_E
Interior exposure	Carbon/epoxy	0.95
	Glass/epoxy	0.75
	Aramid/epoxy	0.85
Exterior exposure	Carbon/epoxy	0.85
	Glass/epoxy	0.65
	Aramid/epoxy	0.75
Aggressive environment	Carbon/epoxy	0.85
	Glass/epoxy	0.5
	Aramid/epoxy	0.7

$$f_{fu} = C_E f_{fu}^*$$

Strength

$$\epsilon_{fu} = C_E \epsilon_{fu}^*$$

Elongation

$$E_{fu} = \frac{C_E f_{fu}^*}{C_E \epsilon_{fu}^*} = E_{fu}^*$$

Modulus

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CSA S806 Environmental Durability Tests

Test	Specification	Condition	Duration	% Retention	
				1000h	3000h
Water Resistance	ASTM D2247 ASTM E104	100% 38 ± 1 °C	1000, 3000, and 10000	90	85
Saltwater Resistance	ASTM D1141 ASTM C581	Immersion at 23 ± 1 °C	1000, 3000, and 10000	90	85
Alkali Resistance	ASTM C581	Immersion in Ca(OH) ₂ at pH=9.5 and 23 ± 1 °C	1000 and 3000	90	85
Dry Heat Resistance	ASTM D3045	60 ± 3 °C	1000 and 3000	90	85

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Strengthening and Repair of Concrete Structures

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FRP Strengthening Limit

ACI 440.2R:

- Loss of FRP should not result in member failure

$$(\phi R_n)_{existing} \geq (1.1S_{DL} + 0.75S_{LL})_{new}$$

“Supplemental Reinforcement”

Existing strength without FRP should be sufficient to support typical service loads

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FRP Strengthening Limit

CSA S806:

- Loss of FRP should not result in member failure

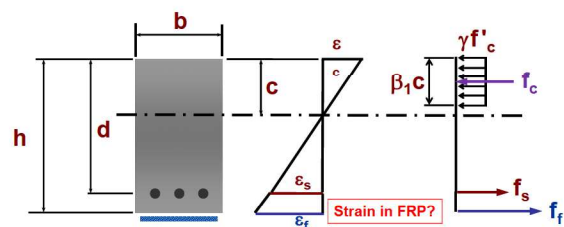
$$(\phi R_n)_{existing} \geq (1.0S_{DL} + 1.0S_{LL})_{new}$$

“Supplemental Reinforcement”

Existing strength without FRP should be sufficient to support service loads

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FRP Flexural Design Concepts



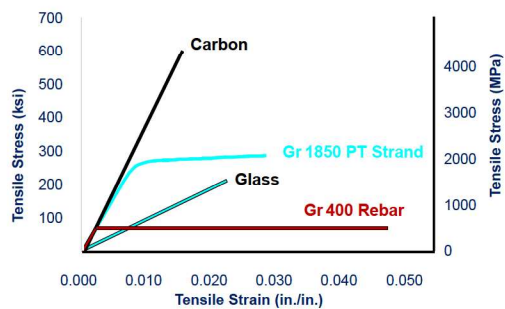
$$M_n = A_s f_s \left(d - \frac{\beta_1 c}{2} \right) + \overset{= 0.85}{\psi_f} A_f f_f \left(h - \frac{\beta_1 c}{2} \right)$$

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Mechanical Properties of FRP

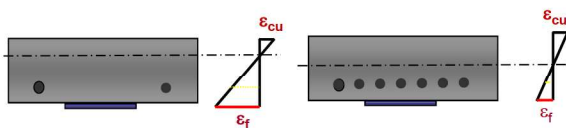


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FRP General Design Concepts

Effectiveness of FRP

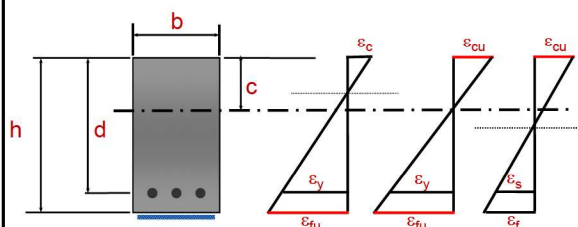
Strain compatibility design approach



FRP design strength can be 50% of published ultimate strength ($0.5 f_{fu}$)

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Failure Modes



Failure mode and strain distribution depends on type and amount of FRP reinforcement

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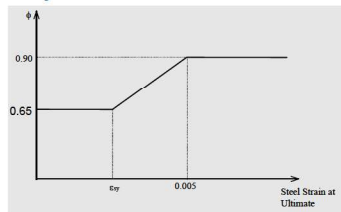
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Strength vs. Ductility

ACI 318:

A section with lower ductility should compensate with a higher reserve of strength



$$\phi = \begin{cases} 0.90 & \text{for } \epsilon_s \geq 0.005 \\ 0.65 + \frac{0.20(\epsilon_s - \epsilon_{sy})}{0.005 - \epsilon_{sy}} & \text{for } \epsilon_{sy} < \epsilon_s < 0.005 \\ 0.65 & \text{for } \epsilon_s \leq \epsilon_{sy} \end{cases}$$

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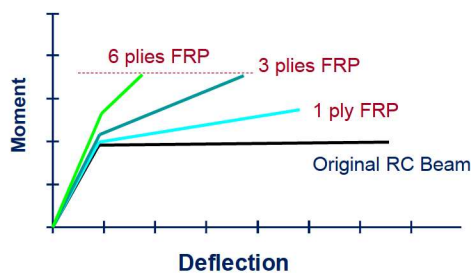
Strength vs. Ductility

CSA S806-12 Cl. 11.2.1:

Strengthening of a member shall not result in the transformation of a ductile failure mode of the unstrengthened member to a brittle failure mode of the strengthened member

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Effect of Adding FRP Plies



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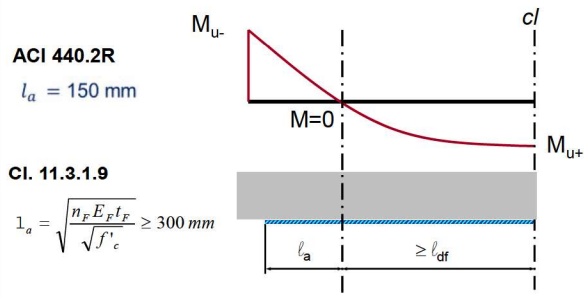
Strengthening and Repair of Concrete Structures

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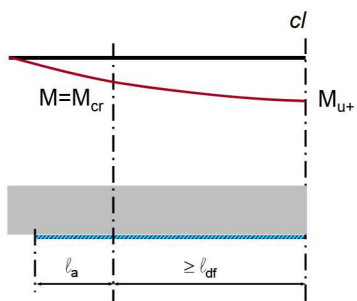
Interfacial Shear Failure – Concrete Cover Delamination



Cut-off Points: Continuous Span

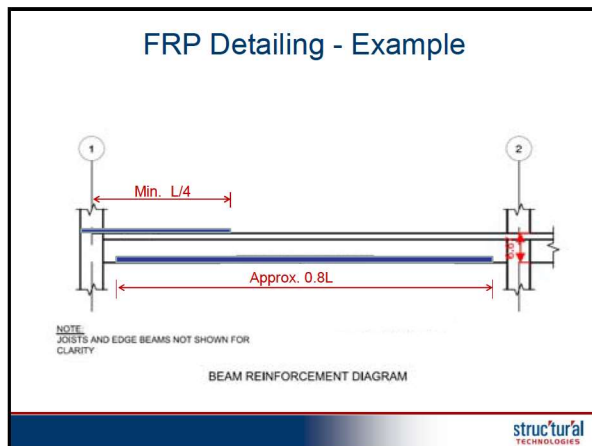


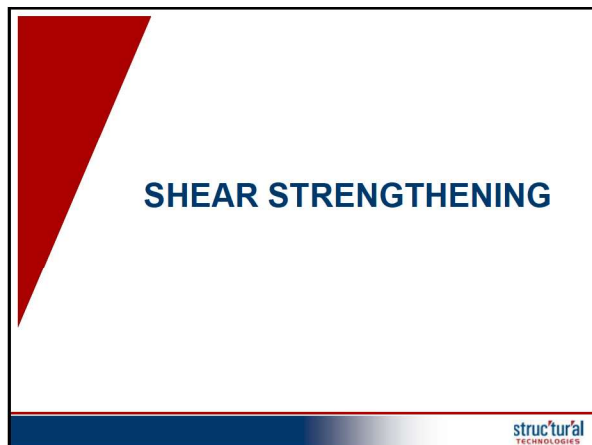
Cut-off Points: Simple Span

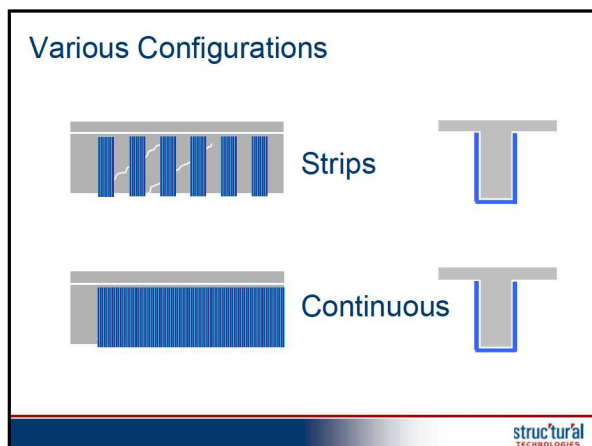


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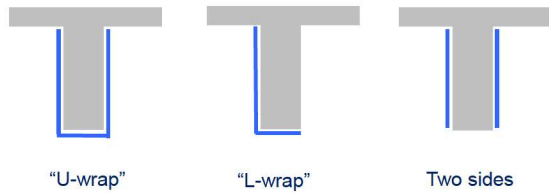




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Wrapping Schemes

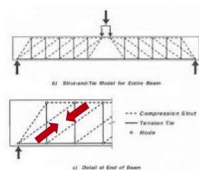


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Shear Strength

$$V_n = V_c + V_s + \psi_f V_f$$

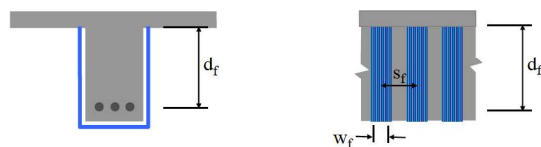
$$V_s + V_f \leq 0.66 \sqrt{f'_c} b_w d$$



$\psi_f = 0.95$	Completely wrapped members
$\psi_f = 0.85$	Three-sided U-wraps or bonded face plies

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FRP Contribution



$$V_f = \frac{A_{fv} f_{fe} d_f}{s_f}$$

$$f_{fe} = \kappa_v f_{fu} \leq 0.004 E_f$$

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Strengthening and Repair of Concrete Structures

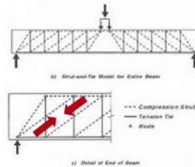
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Shear Strength

CSA S806-12 Cl. 11.3.2.5

- The factored shear resistance (V_r) of the retrofitted beam

$$V_r = V_c + V_s + V_F$$



d_v : Effective shear depth for internal steel equal to 0.9d or 0.72h, whichever is greater

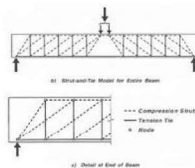
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Shear Strength

CSA A23.3-19

$$V_s = \frac{\phi_s A_v f_y d_v}{s}$$

$$V_c = \phi_c \lambda \beta \sqrt{f'_c} b_w d_v$$



d_v : Effective shear depth for internal steel equal to 0.9d or 0.72h, whichever is greater

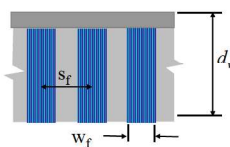
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Shear Strength

$\phi_c = 0.65$
 $\phi_s = 0.85$
 $\phi_F = 0.65$

CSA S806-12 Cl. 11.3.2.6

$$V_F = \frac{\phi_F A_F E_F \epsilon_F d_v}{S_F}$$



$$f_F = E_F \epsilon_F$$

d_v : Effective shear depth for internal steel equal to 0.9d or 0.72h, whichever is greater

α_F : The orientation angle of the fibers with respect to the longitudinal axis of the member

θ : Acute angle of fiber direction to member axis

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Effective FRP strain

$$\varepsilon_F = 0.006 \leq 0.75 \varepsilon_{Fu}$$

Fully-wrapped

$$\varepsilon_F = 0.005 \leq 0.75 \varepsilon_{Fu}$$

U-wrapped with proven anchorage system

$$\varepsilon_F = \kappa_v \varepsilon_{Fu} \leq 0.004$$

U-wrapped without anchoring or side bonded

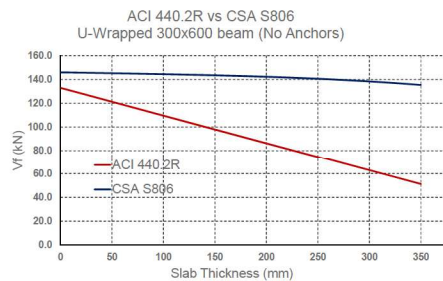
Limit for debonding failure mode Limit for loss of aggregate interlock

$$\kappa_v = \frac{k_1 k_2 L_e}{11,900 \varepsilon_{fu}} \leq 0.75$$

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CSA S806-12 vs. ACI 440.2R-17

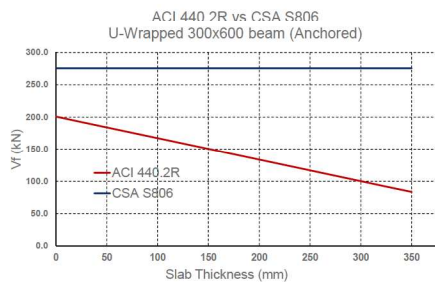
- Shear strengthening of beams (no FRP anchors)



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CSA S806-12 vs. ACI 440.2R-17

- Shear strengthening design of beams (with FRP anchors)



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Shear strengthening limitations

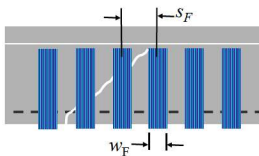
CSA S806-12 Cl. 11.3.2.3, 11.3.2.4:

Beams with total depth less than 300 mm shall not be strengthened for shear unless fully wrapped on all four faces or a proven anchorage system is used to develop the design strength of the FRP

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Shear Strengthening - Detailing

Spacing Requirements



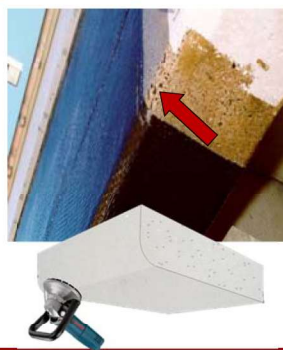
$$s_F \leq \begin{matrix} w_F + 0.25d_v \\ w_F + 300mm \end{matrix}$$

FRP strips shall be placed between steel stirrups rather than over the steel stirrups

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Shear Strengthening - Detailing

- Minimum corner radius for shear strengthening is not defined for beams
- Minimum corner radius is 12 mm (ACI 440.2R)
- Minimum corner radius for columns is 20 mm (Cl. 11.4.2.2)



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PARKING GARAGE
PRECAST DOUBLE TEE AND CORBEL
STRENGTHENING

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Existing Conditions



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Precast Tee FRP Strengthening



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Precast Tee FRP Strengthening



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Corbel FRP Strengthening



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Corbel FRP Strengthening

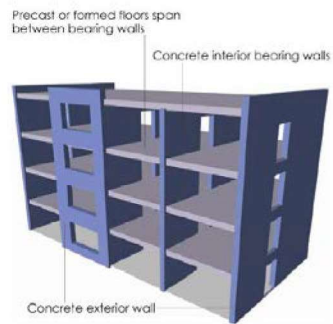


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Shear Walls



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Shear Walls

Diagonal Shear Failure



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Shear Walls Shear Strengthening

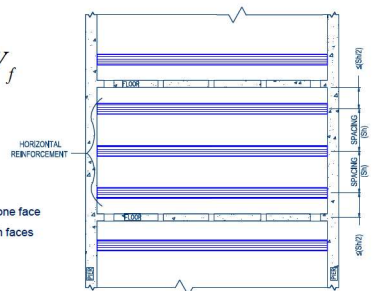
$$V_n = V_c + V_s + \psi_f V_f$$

$$V_f = m n_f t_f f_f D$$

$m = 0.75$ for walls strengthened on one face

$m = 2$ for walls strengthened on both faces

D' = Wall length



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Shear Walls Strengthening

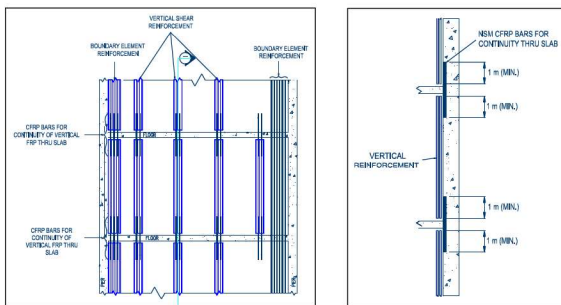
FRP solution



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Shear Walls Strengthening

FRP solution



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Shear Walls Strengthening

FRP solution



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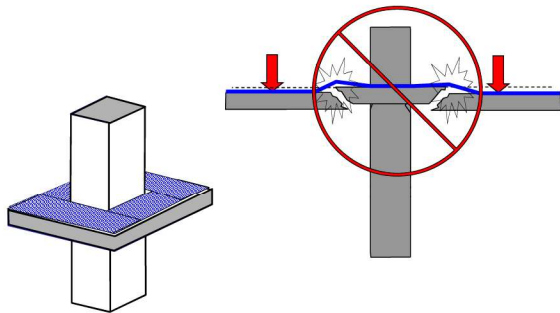
Shear Walls Strengthening

Surface Embedded FRP Bars



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FRP Applications - Punching Shear?



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Confinement



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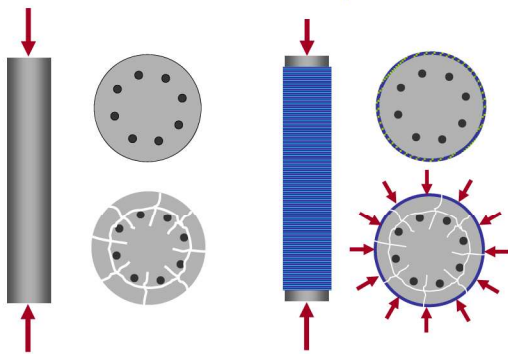
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Column Hinge Failure



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General Concept



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FRP Confined Column



Cylinder ID	V-Wrap C200H
F-0	--
F-1	1 layer
F-2	2 layers
F-4	4 layers

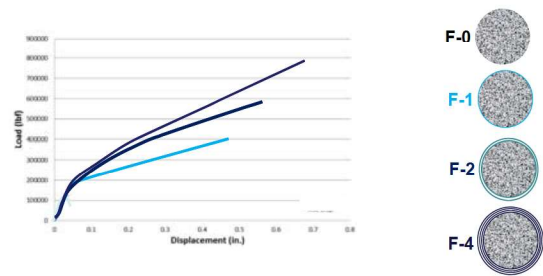


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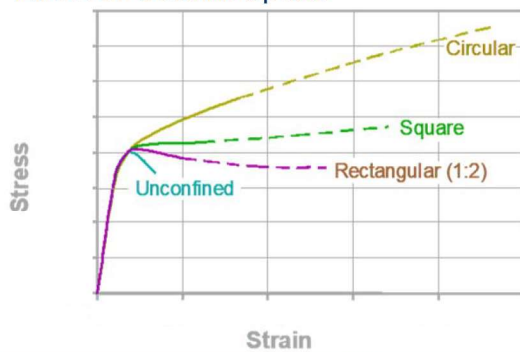
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FRP Confined Column



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Effect of Column Space



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Axial Strength

- ACI 318

$$\phi P_n = 0.85\phi \left[0.85f_{cc}'(A_g - A_{st}) + f_y A_{st} \right]$$

$$\phi P_n = 0.80\phi \left[0.85f_{cc}'(A_g - A_{st}) + f_y A_{st} \right]$$

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Strengthening and Repair of Concrete Structures

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Axial Strength

CSA A23.3

Spirally reinforced columns

$$P_{r,\max} = 0.90 P_{ro}$$

Tied columns

$$P_{r,\max} = (0.2 + 0.002h) P_{ro} \leq 0.80 P_{ro}$$

$$P_{ro} = \alpha_1 \varphi_c f'_{cc} (A_g - A_{st}) + \varphi_s f_y A_{st}$$

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Column Confinement

$$f'_{cc} = f'_c + 3.3 \psi_f \kappa_a f_l$$

$$f_l = \frac{2 E_f n t_f \varepsilon_{fe}}{D}$$

$$\psi_f = 0.95$$

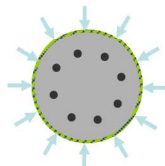
$$\varepsilon_{fe} = \kappa_e \varepsilon_{fu} \quad \kappa_e = 0.55$$

$$\kappa_a = 1$$

$$\kappa_b = 1$$

$$\varepsilon_{ccu} = \varepsilon'_c \left(1.5 + 12 \kappa_b \frac{f_l}{f'_c} \left(\frac{\varepsilon_{fe}}{\varepsilon'_c} \right)^{0.45} \right) \leq 0.01$$

Strain limit to prevent excessive cracking and loss of concrete integrity



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Column Confinement

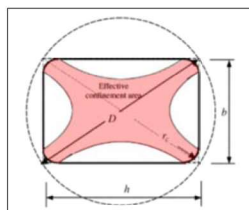
$$f'_{cc} = f'_c + 3.3 \psi_f \kappa_a f_l$$

$$\kappa_a = \frac{A_e}{A_c} \left(\frac{b}{h} \right)^2$$

$$\kappa_b = \frac{A_e}{A_c} \left(\frac{h}{b} \right)^{0.5}$$

$$\frac{A_e}{A_c} = \frac{1 - \left[\left(\frac{b}{h} \right) (h - 2r_c)^2 + \left(\frac{h}{b} \right) (b - 2r_c)^2 \right]}{3bh} - \rho_s$$

$$\varepsilon_{ccu} = \varepsilon'_c \left(1.5 + 12 \kappa_b \frac{f_l}{f'_c} \left(\frac{\varepsilon_{fe}}{\varepsilon'_c} \right)^{0.45} \right) \leq 0.01$$



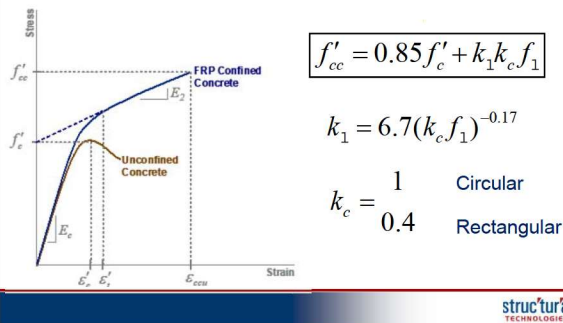
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Strengthening and Repair of Concrete Structures

Toronto, February 21, 2020

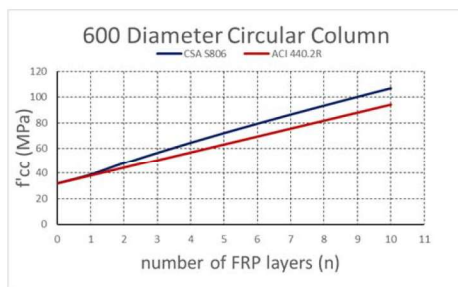
Column Confinement (CSA S806)

Confined Compressive Strength of Concrete (f'_{cc})



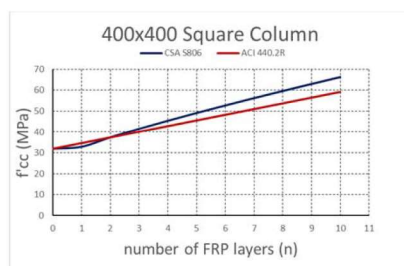
CSA S806-12 vs. ACI 440.2R-17

- Confinement of a circular column using FRP under pure axial load



CSA S806-12 vs ACI440.2R-17

- Confinement of a square column using FRP under pure axial load

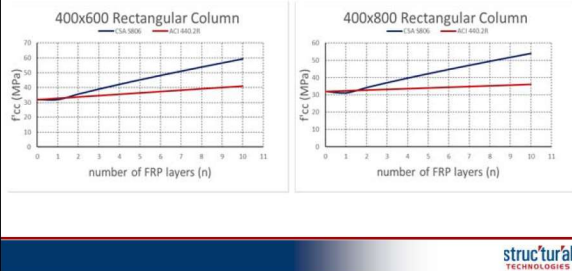


Strengthening and Repair of Concrete Structures

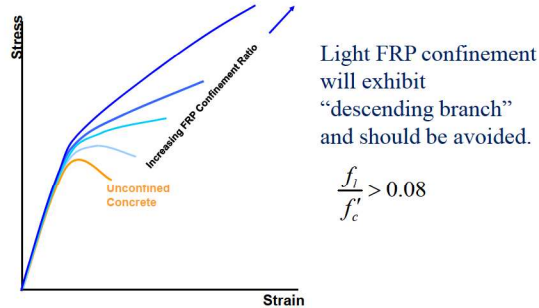
Toronto, February 21, 2020

CSA S806-12 vs. ACI 440.2R-17

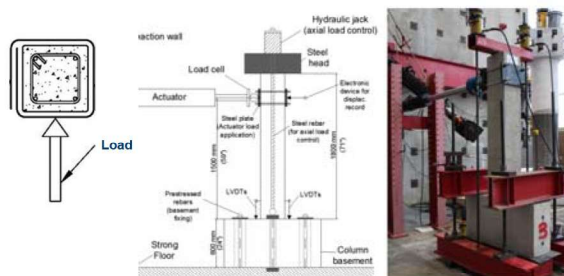
- Confinement of a rectangular column using FRP under pure axial load



Column Confinement



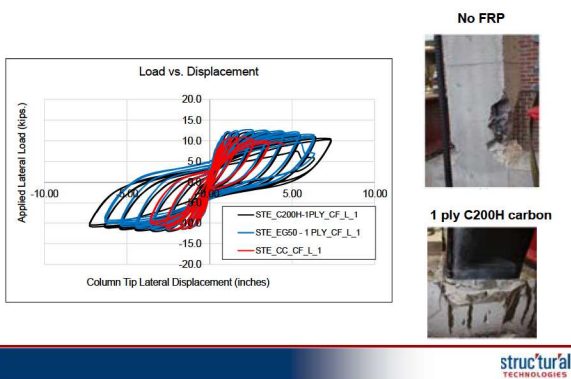
Column Plastic Hinge Confinement



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Column Plastic Hinge Confinement



Column FRP Shear Strengthening



ACI 440.2R Shear Design Method

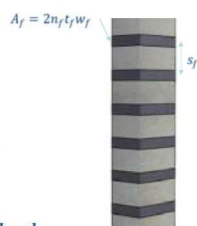
$$V_n = V_c + V_s + V_f$$

FRP design strain: $\epsilon_{fe} = 0.004 \leq 0.75\epsilon_{fu}$

FRP effective stress: $f_{fe} = E_f \epsilon_{fe}$

FRP nominal Shear Strength: $V_f = \frac{A_f}{s_f} f_{fe} d_f$

$$V_n \leq (V_c + 8\sqrt{f'_c} b_w d) \approx 10\sqrt{f'_c} b_w d$$

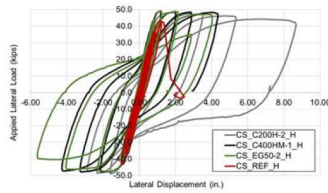


Strengthening and Repair of Concrete Structures

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Column FRP Shear Tests

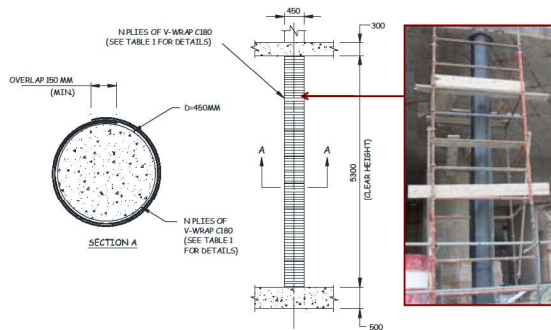
Full Scale Tests



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Landmark Plaza Rehabilitation

Repair Details – Column FRP



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FRP Quality Control

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FRP Quality Control

- Precautions before installation
- Surface preparation
- Detailing
- Proper saturation of FRP sheet
- Quality assurance testing for
 - Intimate contact/bond
 - Bond strength
 - FRP tensile properties

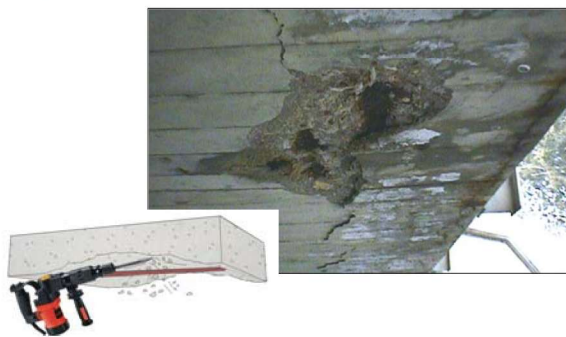
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Installation Precautions...



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Repair Substrate!

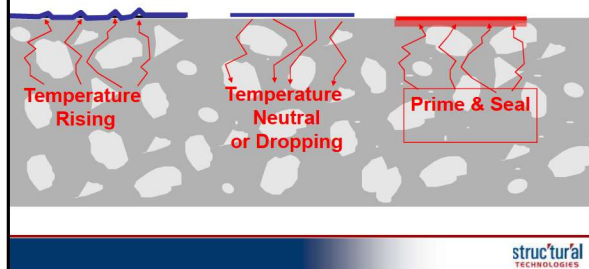


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Strengthening and Repair of Concrete Structures

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Gassing - Prevention and Precaution



Surface Prep - Methods and desired surface

- Abrasive Blast
- Dustless Grinding & Vacuum
- Water Blast & Dry



CSP 3

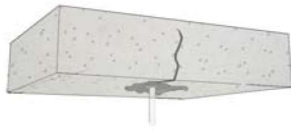


Strengthening and Repair of Concrete Structures

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Inject Cracks

- Crack Repair – Epoxy inject cracks larger than 0.25 mm



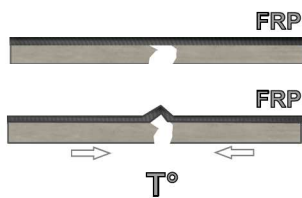
Abrasive Blast
Makes Cracks
Visible!



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FRP Failure Due to Improper Crack Repair

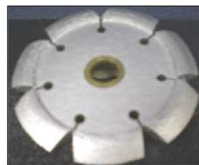
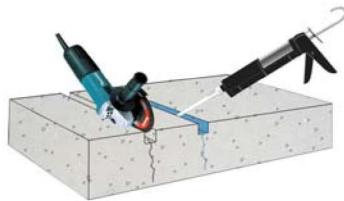
- FRP failure due to improper crack repair



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Slab Crack Repair – Grout Crack & Gravity Feed

- Rout and seal cracks larger than 0.25 mm



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If Fiber Direction Is Perpendicular to Corner



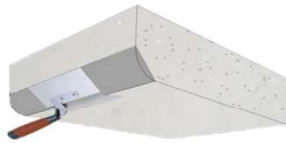
Column corners



Beam corners

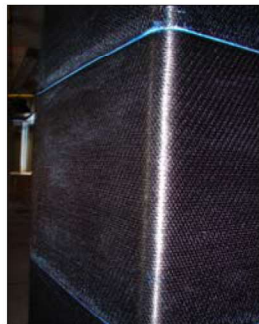
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Rounding of Corners



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FRP Over Rounded Corner on Column

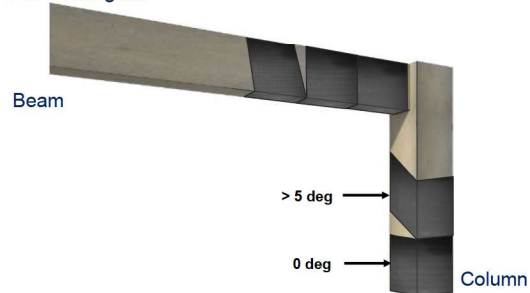


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If Corner Is Not Evenly Rounded...

FRP Misaligned



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Quality Control – Saturation Process



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FRP Sheet Installation



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Sounding Bond Tests



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Quality Control - Delamination Test



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Small Delamination Repair Procedure

Insert resin into delamination

- Less than 13 cm² or 5% Total Area Max
 - OK. No repair required
- If 13 to 30 cm²
 - Inject with low pressure



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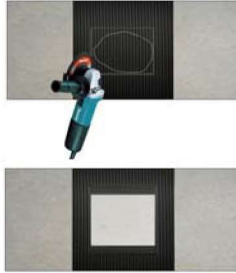
Strengthening and Repair of Concrete Structures

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Large Delamination Repair Procedure

Remove/Replace FRP:

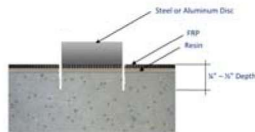
- If greater than 30cm² grind or sawcut area
- Replace w/equivalent sheets
- Minimum repair lap = 150mm



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FRP Pull-off Test

Acceptance Criteria - Minimum bond = 1.4 MPa (200 psi)



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Bond Test Location and Frequency

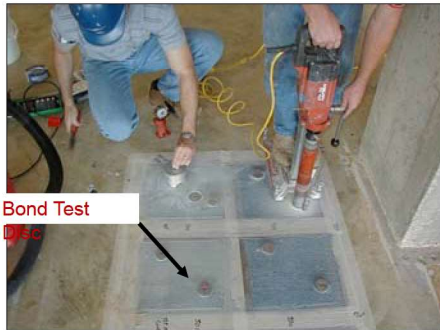
1 PULL PER INSTALLATION DAY MINIMUM OR
PER ENGINEER'S SPEC (per m² installed)



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Pre-Installation Pilot Bond Pull-off Testing?



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QA Procedures for FRP Properties

Material Properties

- Witness panels to evaluate strength, strain, & modulus
- Test panels from field materials
- ASTM D3039 test method

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FRP QA/QC Witness Panel Test



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Strengthening and Repair of Concrete Structures

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Tensile Test – ASTM D3039

1.2. TEST RESULTS

Table 1.2 – Tensile Test Results for C400HM fabric with V-Wrap 770 resin, per ASTM D3039

Specimen ID	Average width, in.	Area, sq. in.	Peak load, kN	Load/width, kN/m	Strength, MPa	Modulus, GPa	
	mm	in.	kN	lb/in	ksi	GPa	
C400HM_TN01_01	22.58	0.889	43.88	0.071	71.23	16000	5.15
C400HM_TN01_02	21.26	0.837	43.20	0.067	71.39	16042	3.36
C400HM_TN01_03	22.68	0.895	46.09	0.071	72.73	16343	3.21
C400HM_TN01_04	22.66	0.895	46.09	0.071	75.05	17001	3.34
C400HM_TN01_05	22.75	0.899	46.19	0.072	76.41	17171	3.36
Average	22.28	0.881	46.48	0.071	73.48	16613	3.28
St=	0.63	0.025	1.29	0.002	2.42	543	0.10
CVI (%)	2.8	2.8	2.8	5.5	5.5	2.9	2.9
C400HM_TN02_01	22.61	0.890	43.94	0.071	77.45	17405	3.43
C400HM_TN02_02	22.63	0.891	43.99	0.071	69.10	15527	3.05
C400HM_TN02_03	22.53	0.887	45.79	0.071	76.54	17108	3.40
C400HM_TN02_04	22.73	0.895	46.19	0.072	76.61	17215	3.37
C400HM_TN02_05	24.10	0.945	46.58	0.076	79.57	17745	3.28
Average	22.82	0.902	46.68	0.072	75.75	17018	3.38
St=	0.67	0.026	1.35	0.002	3.64	802	0.13
CVI (%)	2.9	2.9	2.9	5.1	5.1	4.6	4.6
C400HM_TN03_01	22.61	0.890	45.94	0.071	75.52	17446	3.47
C400HM_TN03_02	21.82	0.858	44.34	0.069	64.36	14462	2.95
C400HM_TN03_03	22.99	0.905	46.71	0.072	73.97	16623	3.22
C400HM_TN03_04	21.87	0.861	44.44	0.069	71.48	16063	3.27
C400HM_TN03_05	22.20	0.874	45.11	0.070	71.49	16045	3.22
Average	22.38	0.878	45.31	0.070	71.86	16148	3.23
St=	0.50	0.020	1.01	0.002	5.14	1154	0.19
CVI (%)	2.2	2.2	2.2	7.1	7.1	5.8	5.8



*Failure mode based on ASTM D3039, refer to Figure 1.1



Thank You

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TECHNOLOGIES





Workshop:
**STRUCTURAL
STRENGTHENING**

Feb. 21, 2020

Matt Hickey, P.Eng.
Division Manager (Ontario)

Jay Thomas
VP of Strengthening Division

Tarek Alkhrdaji, PhD, P.E.
VP of Engineering



Agenda

- Introduction & Housekeeping
- Differences New Construction vs Strengthening
- FRP
 - Most commonly used FRP applications
 - Installation techniques
 - Design strategy (ACI 440 & CSA S806)
 - FRP limitations & fire protection
 - QC
- Conventional Techniques
- Case Studies
 - Flexure, Shear, Confinement
- PT Repairs
- ACI 562
- Questions....





PULLMAN Specialty Contracting Services

- Strengthening of Existing Structures
- Post Tensioning Reinforcement Systems & Repairs






Strengthening Solutions

Applications


- Adaptive reuse
- New penetrations
- Adding new loads
- Construction defects
- Honeycomb and void repairs
- Damage and overloading
- Blast upgrades



FRP

External PT

Enlargement



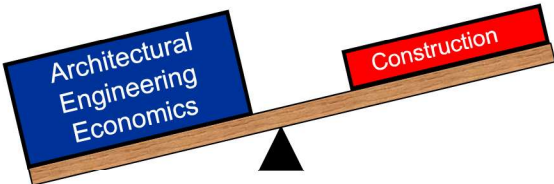
NEW CONSTRUCTION
VS.
STRENGTHENING



The New Construction Process


↓

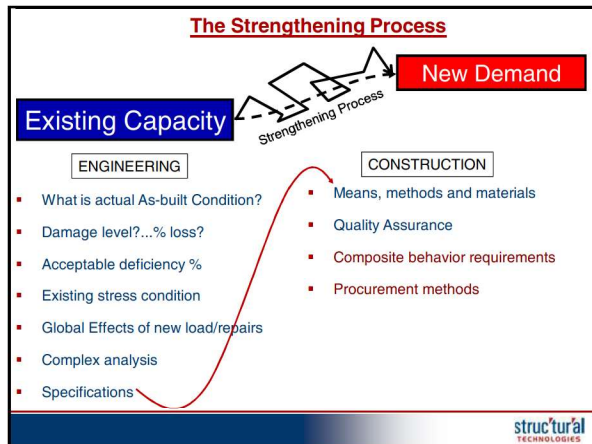
Function and Structural Demand

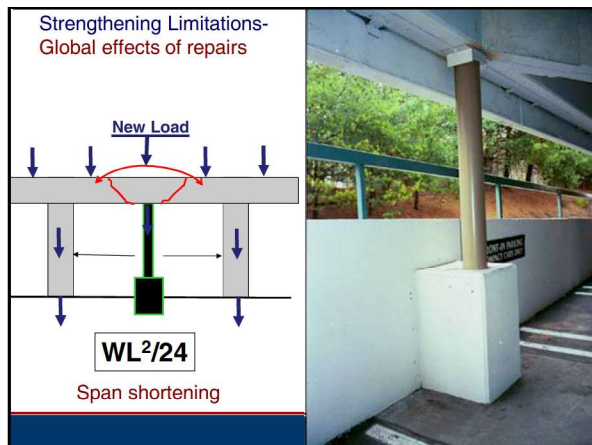


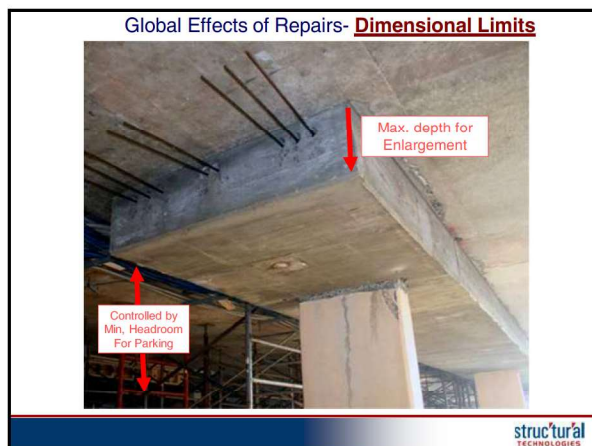
Architectural Engineering Economics

Construction

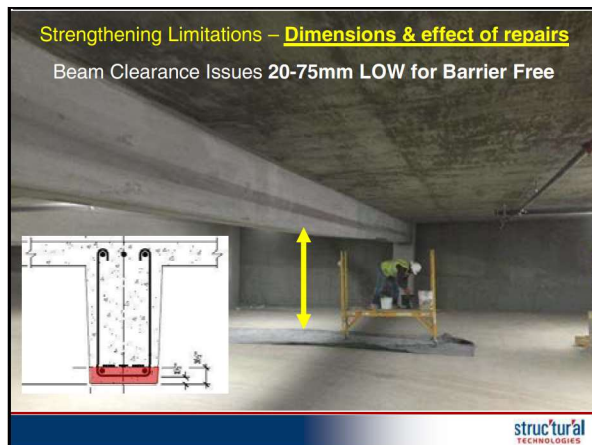










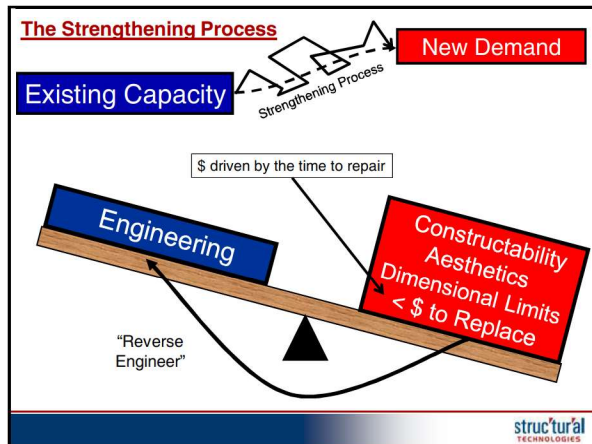






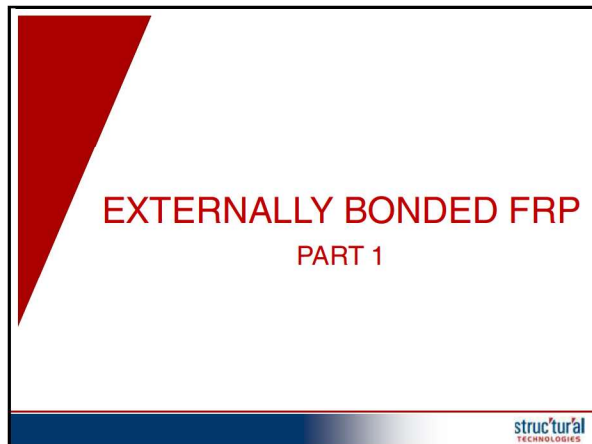






- Strengthening Techniques and Topics for Today**
- Externally bonded FRP
 - Supplemental steel and steel plate bonding
 - Concrete section enlargement
 - External post tensioning
 - Hybrid solutions
 - Features and benefits
 - Limitations...constructability & % increase
 - Design considerations
 - Detailing
 - Fire considerations
 - Case study projects
 - Relative costs
- structural TECHNOLOGIES

- % Use of Strengthening Systems**
- ~50% ▪ FRP Composites
 - ~50%
 - External Post Tensioning
 - External in enlarged section
 - External
 - Internal in drilled/cored holes
 - Enlargement of Section
 - Supplemental Steel (Plate bonding or new supports)
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Span shortening with steel beams

Span shortened only 600mm at each end to use existing foundation

$WL^2/24$

NEW LOAD

2ft

Foundation

Foundation

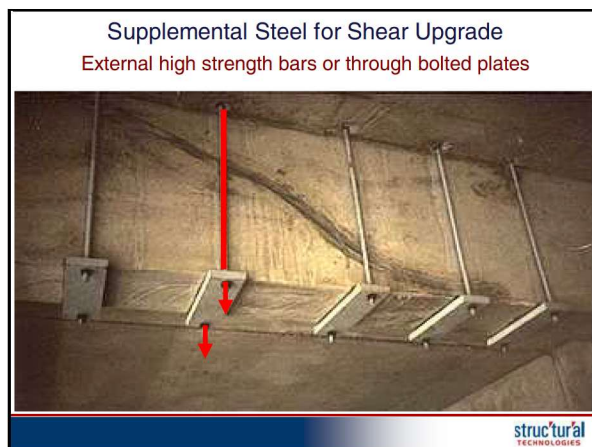


The diagram shows a cross-section of a beam supported by two foundations, each 2ft wide. A new load is applied to the top of the beam. The formula $WL^2/24$ is shown. The photograph shows a new steel beam being installed, with the label 'New Steel Beam'.

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Supplemental Steel for Shear Upgrade

External high strength bars or through bolted plates



The photograph shows external high strength bars or through bolted plates installed on a concrete beam. A red arrow points to the bars.

structural TECHNOLOGIES

Supplemental Steel for Shear Upgrade

Internal high strength bars or rebar

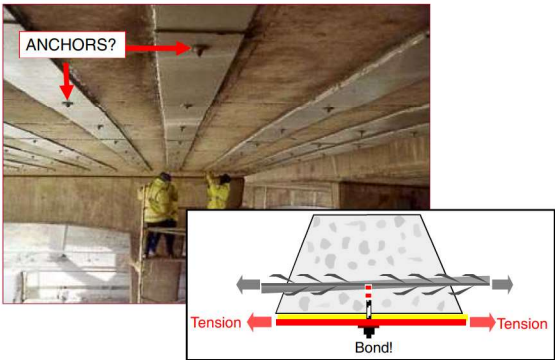
T-Head



The photograph shows internal high strength bars or rebar installed in a concrete beam. The diagram shows a cross-section of a beam with a T-Head and a vertical bar.

structural TECHNOLOGIES

Steel Plate Bonding Concept



The diagram shows a cross-section of a concrete beam with a steel plate bonded to its bottom surface. A red arrow points to the bond interface with the label "Bond!". Below the bond, a red arrow points to the left and a red arrow points to the right, both labeled "Tension". Above the bond, a red arrow points to the right, labeled "Tension". A red arrow points to the top of the concrete beam with the label "ANCHORS?". The photo shows two workers in yellow safety gear working on a concrete ceiling structure.

ANCHORS?

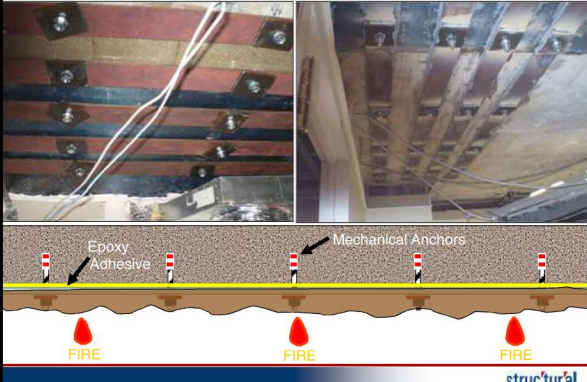
Tension

Bond!

Tension

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Steel Plate Installation "Dry-Fitting" Components



The diagram shows a cross-section of a concrete beam with a steel plate bonded to its bottom surface. The bond is labeled "Epoxy Adhesive". Mechanical anchors are shown in the concrete. Below the bond, three red flames are labeled "FIRE". The photo shows two workers in yellow safety gear working on a concrete ceiling structure.

Epoxy Adhesive

Mechanical Anchors

FIRE

FIRE

FIRE

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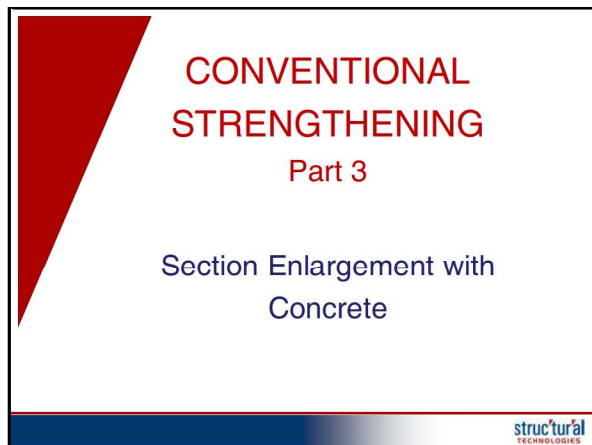
Steel Plate Installation



The photo shows two workers in yellow safety gear working on a concrete ceiling structure. One worker is using a tool to install a steel plate, and the other is assisting. The structure is made of concrete and steel beams.

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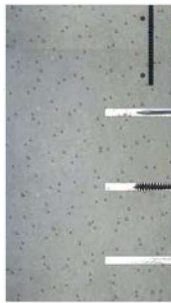


Enlargement- Specifying & Measuring Surface Preparation

- Concrete to Concrete BOND created by:
 1. Horizontal shear transfer
 - a. Through installation of steel dowels
 - b. Profiling of concrete surface (CSP-6)
 2. Surface prep that profiles & opens the concrete pores
 3. A placement techniques to force intimate contact between exiting and new concrete



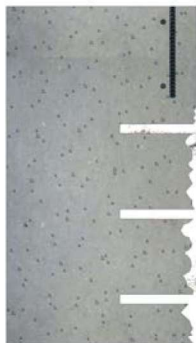
1. Locate Steel, Drill and Clean holes for dowels



- Type of Drill?
- Hole Cleaning?
- Wire Brush
- HALF Pullout Strength results based on drill type and cleaning!

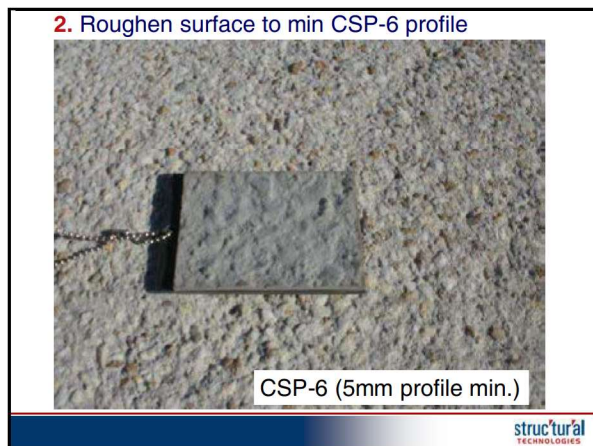


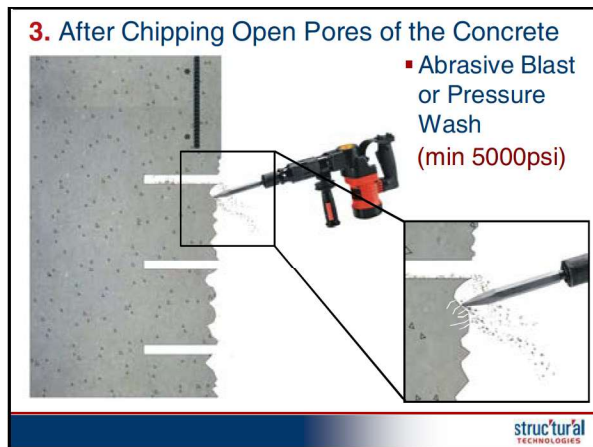
2. Roughen Surface to min 3/16" Profile

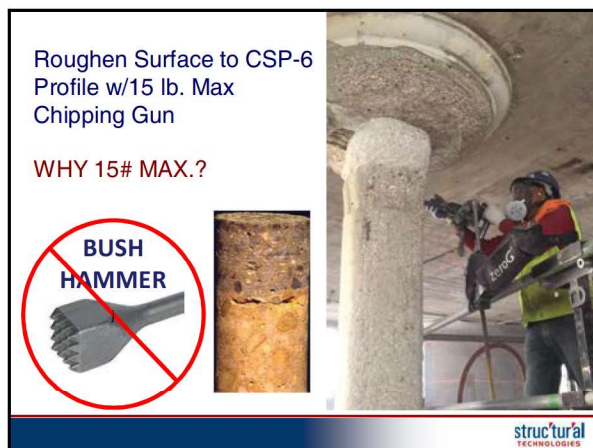


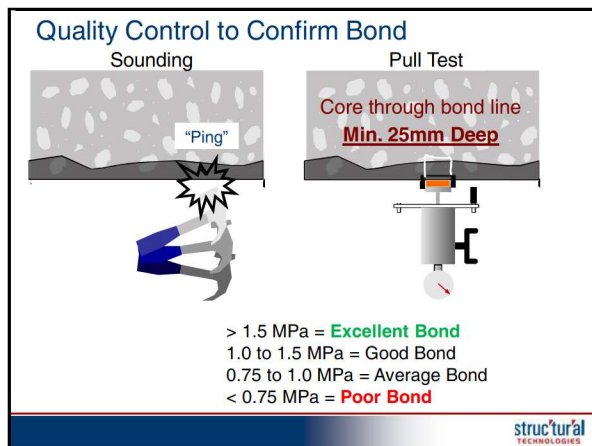
- Chip
- Hydro-Blast (20k psi)
- Spongeblast

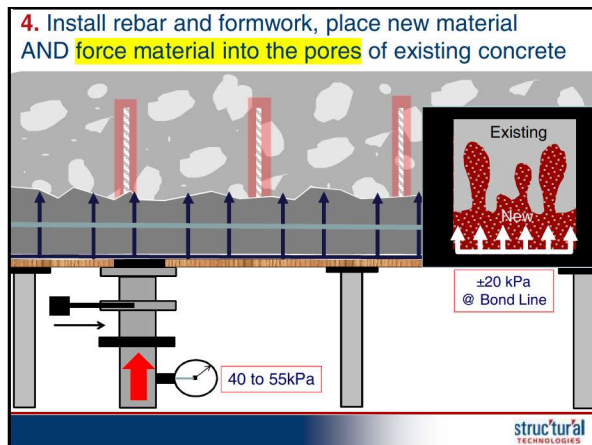














Strengthening solutions Enlargement with Form and Pump pressurized placement for BOND

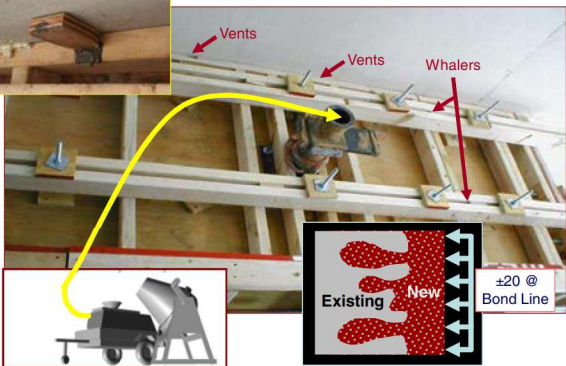
Roughen surface and OPEN PORES!



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Typical F&P Formwork & Pressurization

Vents Vents Whalers




±20 @ Bond Line

Existing New

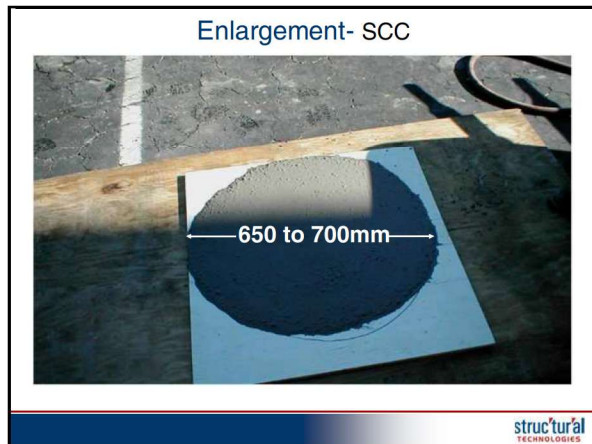
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Strengthening solutions Enlargement with Form and Pump pressurized placement for BOND

Existing MEP left in place



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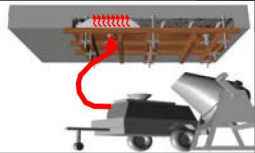



Slab deficient in:
+M, -M & punching shear

Strategies:

1. FRP top and bottom & small PS drop panel
2. Drop capital span/6 for PS, -M and possibly +M?
3. Form & Pump placement

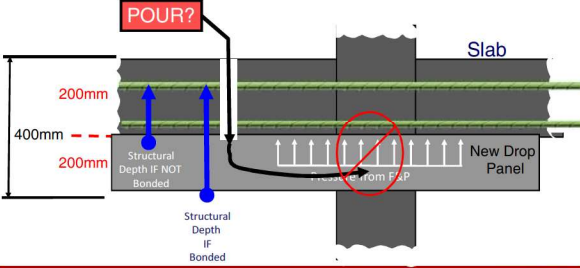







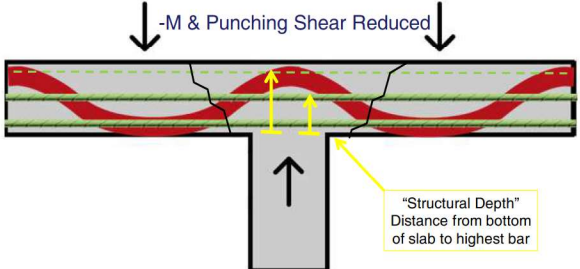
Strengthening Solutions

Structural effect adding drop panels and the importance of bond




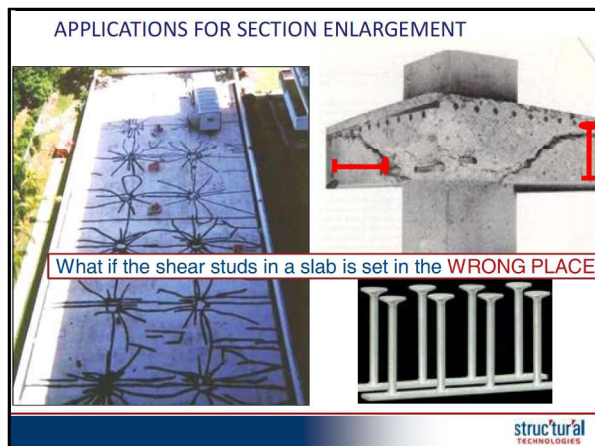


TYPICAL APPLICATIONS FOR DROP PANELS



What if the top steel in a slab is set in the TOO DEEP?










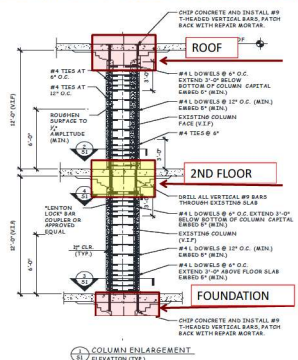
Strengthening solutions Column Enlargement with Form and Pump



Refinery

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Strengthening solutions Column Enlargement w/Continuous Vertical Bar



ROOF

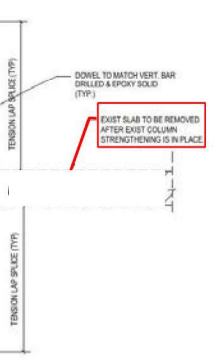

2ND FLOOR

FOUNDATION

COLUMN ENLARGEMENT
ELEVATION (TYP)

struc'tural TECHNOLOGIES

Column Enlargement for Slab Removal- New Atrium



EXIST COLUMN SEE PLAN

PROVIDE ACCESS HOLES IN EXIST SLAB FOR DOWEL

2\" data-bbox="186 796 226 816"/>

4\" data-bbox="206 826 226 846"/>

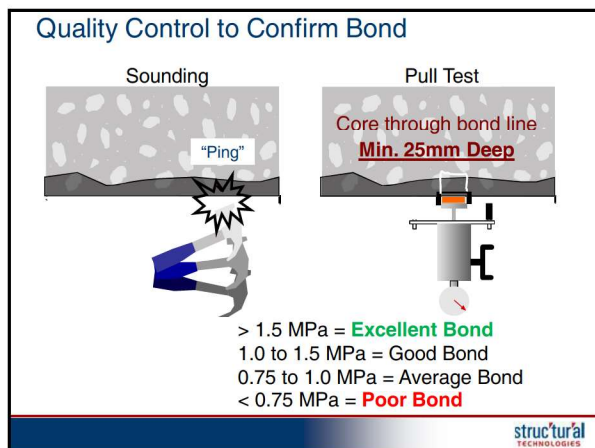
EXIST COLUMN SEE PLAN

EXIST SLAB TO BE REMOVED AFTER EXIST COLUMN STRENGTHENING IS IN PLACE

struc'tural TECHNOLOGIES







Ext Post Tensioning Design and Detail Considerations

- External PT or cast into concrete enlargement? (fire or aesthetics)
- Sizable increase in capacity
- Tendon draped or deviated at 1 or 2 points?
- Anchorage detailing critical
- Excessive uplift forces (reverse)
- Sequential loading/stressing

struc'tural
TECHNOLOGIES

Strengthening Solutions Slab External Post Tensioning

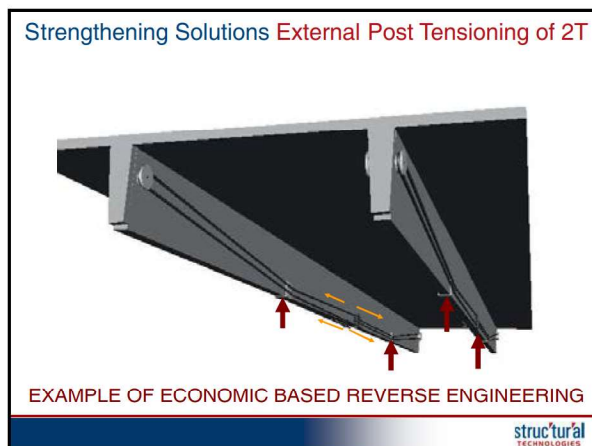


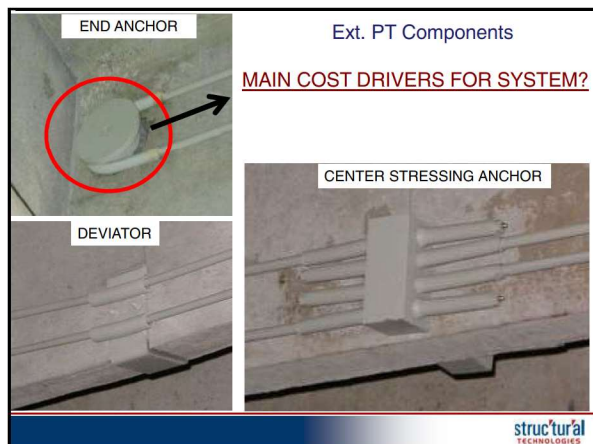
struc'tural
TECHNOLOGIES

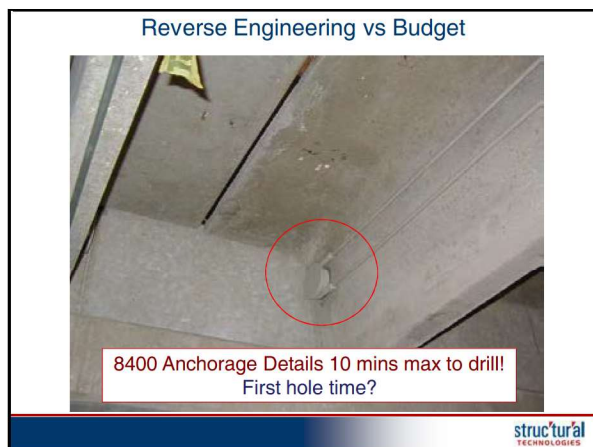
Strengthening Solutions External Post Tensioning

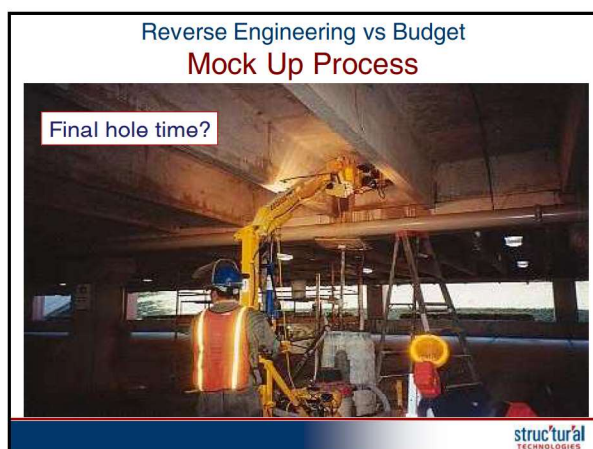


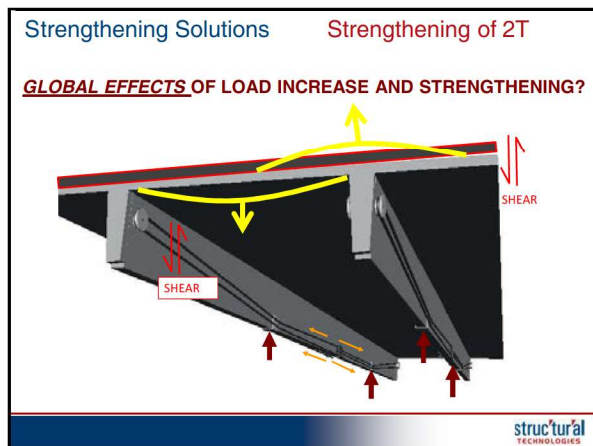
struc'tural
TECHNOLOGIES





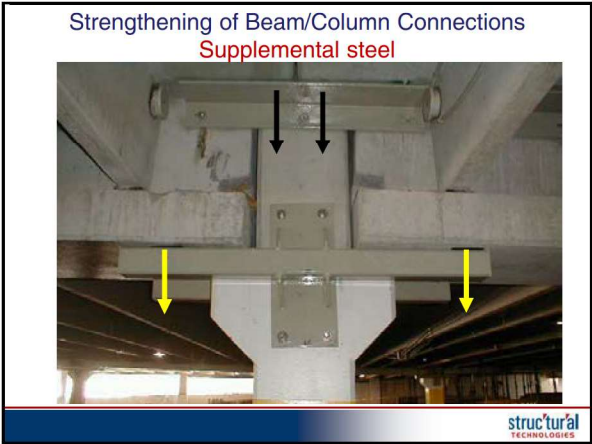








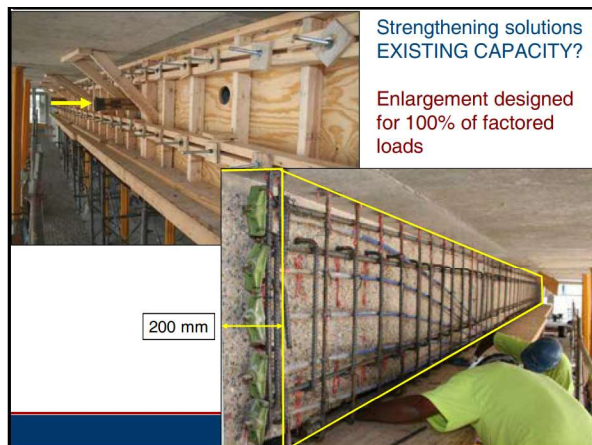




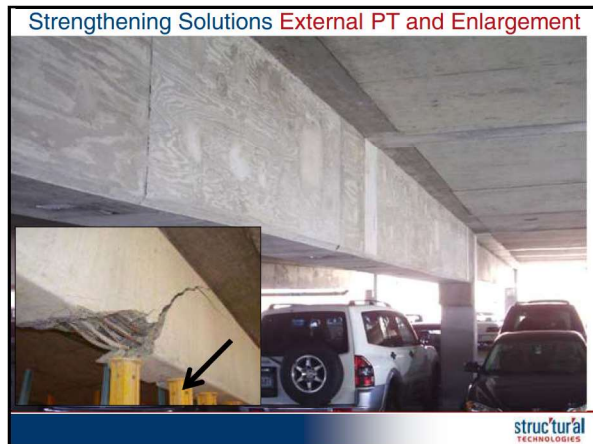






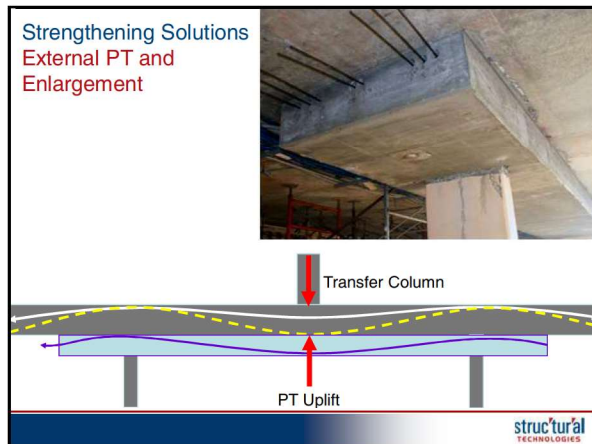




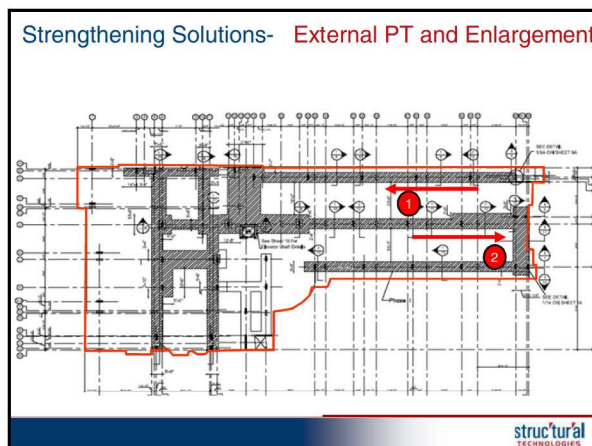












Strengthening Solutions- External PT and Enlargement



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TECHNOLOGIES

Strengthening Solutions- External PT and Enlargement



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TECHNOLOGIES

Strengthening Solutions- External PT and Enlargement
Window in F&P formwork for placement confirmation

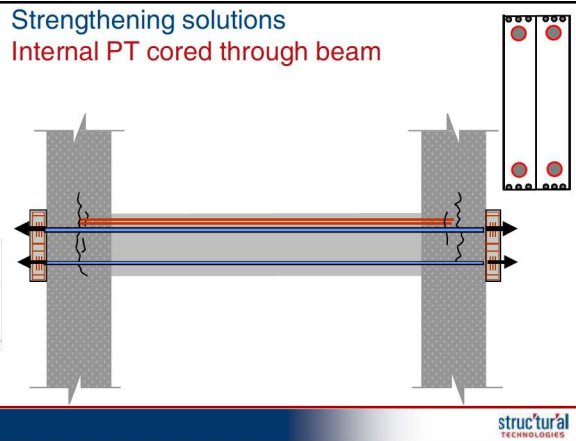


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TECHNOLOGIES

Strengthening Solutions- External PT and Enlargement
Vents in F&P formwork for placement confirmation



Strengthening solutions
Internal PT cored through beam

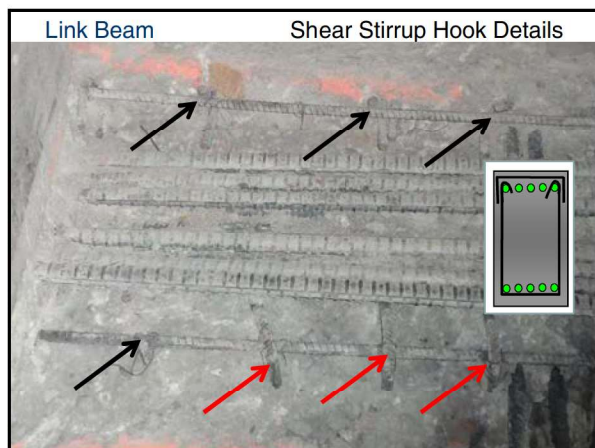


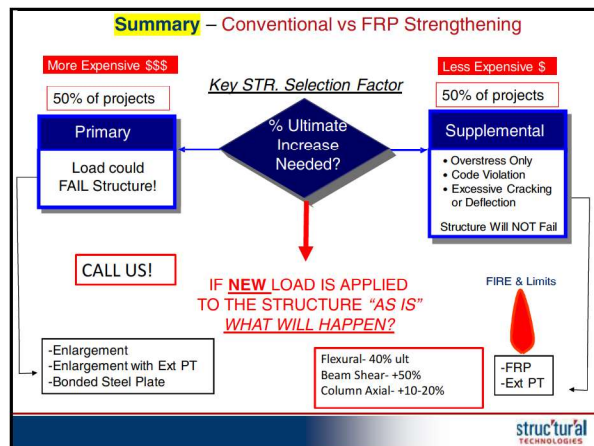
Strengthening solutions internal
PT cored through beam

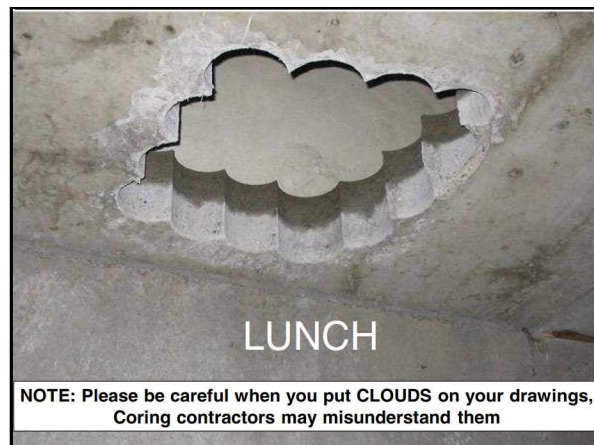












Types of Strengthening Specifications

- Prescriptive...
Supply step by step procedures, materials and quantities, etc.
- ★ Performance...
-Supply *Existing Capacity and New Demand*.
Strengthening design, materials, constructability, etc. are considered to optimize options and costs
-Attracts experienced, qualified specialty contractors

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THANK YOU!


- Contracting Services → **PULLMAN**
- Specialty Products
- Design Support
- Estimate Support
- Forensic Support
- Specifications and Material Selection

→ **struc'tural**
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FRP, Performance, Form and Pump


- Lunch and Learn version available
- Matt Hickey- mhickey@pullman-services.net

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*Post-Tensioning Concepts, Repairs, Modifications and
Evaluation of Existing PT Structures*

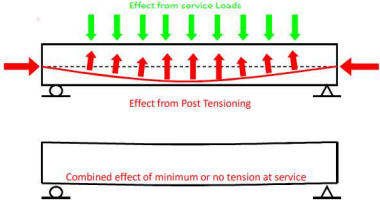
Jay Thomas
VP Strengthening Division
Structural Technologies



Basics of Post-Tension
Prestressed Concrete



Effect of Post Tensioning Combined Effect at Service Loads




Effect from service Loads

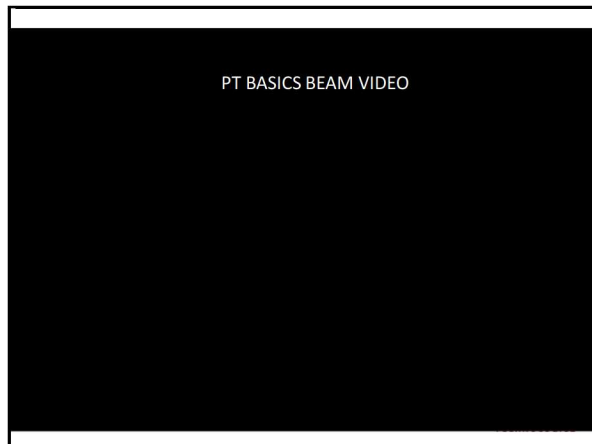
Effect from Post Tensioning

Combined effect of minimum or no tension at service

"LOAD BALANCING"

◀ PREV BASICS NEXT ▶







Post-Tensioned Concrete Advantages

Engineering AND Cost Savings

- Design flexibility- LONGER SPANS- LESS COLUMNS
- Structural depth- THINNER, LIGHTER ELEMENTS and STRUCTURE, FOUNDATION LOADS REDUCED
- Controls deflection- SLABS REMAIN UNCRACKED UNDER SERVICE LOADS
- Effective use of high strength concrete
- Height savings- MORE FLOORS/SAME HEIGHT
- Reduces shoring and re-shoring time- AFTER STRESSING, FORMS PULLED



Typical Prestressing Steel Used for Post Tensioning



Buttonhead Wire

- Ultimate Strength 240ksi
- 0.25" diam



High Strength 7-wire strand

- Ultimate Strength 270 ksi
- Primarily 0.50"- 0.60" diam



Bar Systems

- High Strength
- Threaded



Evolution of Unbonded PT Systems

1966 Buttonhead wire in paper sheathing 1960 Strand in loose plastic sheathing 1980 Strand in extruded plastic sheathing 1985 Present Initial encapsulated tendon system



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TECHNOLOGIES

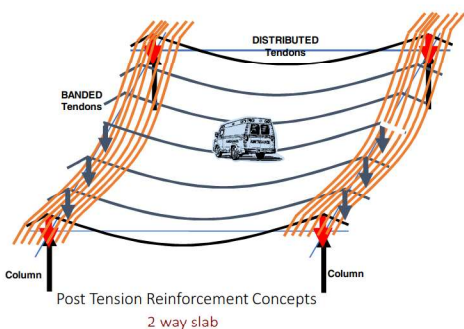
Evolution of Unbonded PT Sheath Systems

moisture moisture moisture moisture

Paper Wrapped Push Through Heat Sealed Extruded (with Grease & Sheathing)

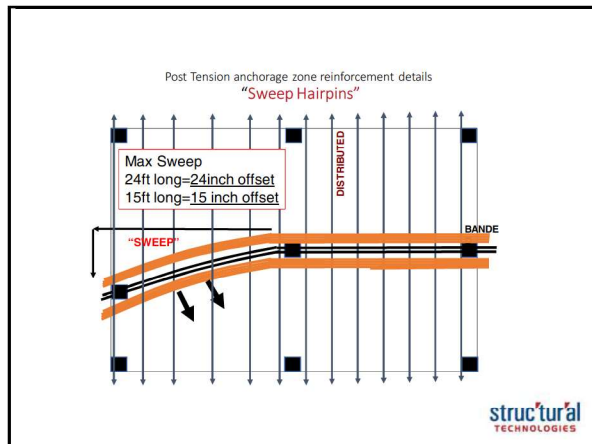


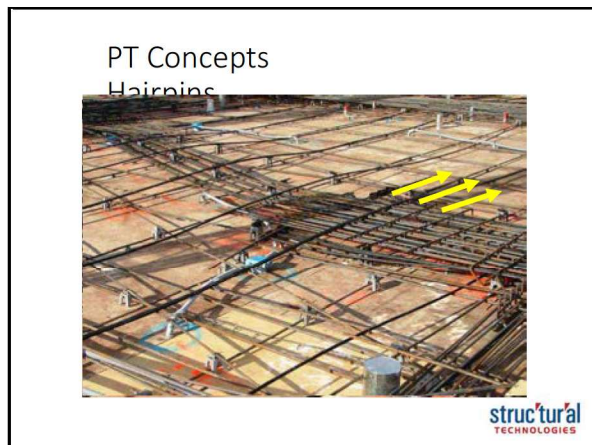
structural
TECHNOLOGIES

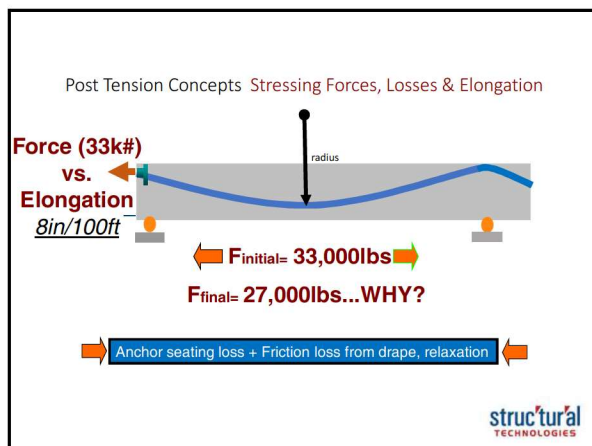


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QC Stressing Logs to Confirm Uniform Force in Strands Confirms proper Elongation vs Force=



8008 HAUTE COURT, SPRINGFIELD, VA 22150
PHONE: 703/451-4300 FAX: 703/451-0862

MONOSTRAND STRESSING RECORD

JOB NAME: Wilton House

LOCATION: Arlington VA

FLOOR: 1B

JOB NO.: 6/22/1905

DATE: 1/1/2004

GAUGE NO.: 2/28/1914

POUR: 1

RAM NO.: 5173

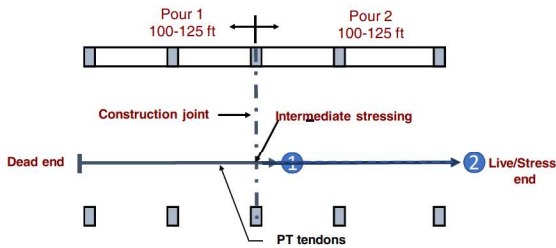
STRESSED BY

VERIFIED BY

TENDON NO.	GAUGE PRESSURE		CALC. ELONG.	MEASURED ELONGATION			% DEV.
	JACKING	LIFT OFF		END 1	END 2	TOTAL	
1	7000		7 1/4	7 1/2		7 1/2	3%
2	7000		7 1/4	7 7/8		7 7/8	8%
3	7000		7 1/4	7 1/4		7 1/4	0%

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Post Tension Reinforcement Concepts- Pour Sequence Intermediate Stressing

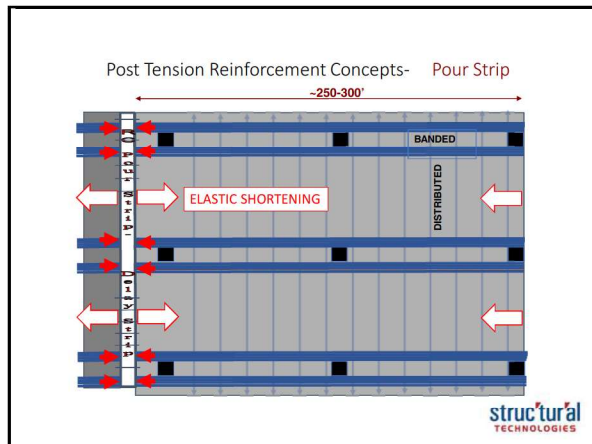


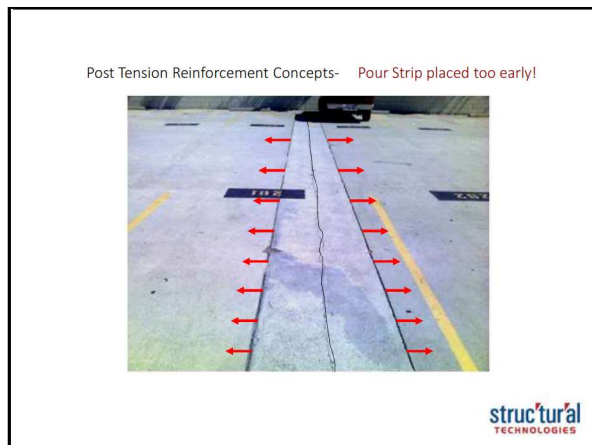
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TECHNOLOGIES

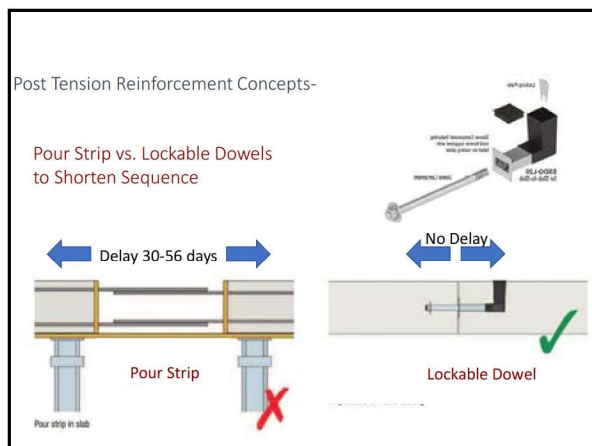
Post Tension Reinforcement Concepts- Intermediate Stressing



structural
TECHNOLOGIES



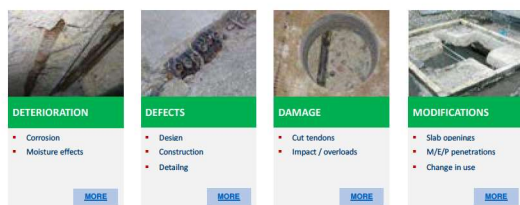




Common Problems with Post-Tensioned Concrete



Common Problems with Post-Tensioned Concrete



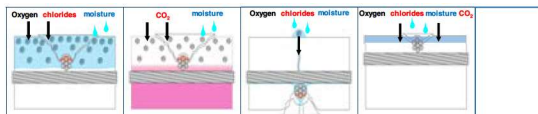
Common PT Problems: Corrosion of tendons and anchors



[PREV](#) [PROBLEMS](#) [NEXT](#)



Common PT Problems: Causes of corrosion on tendons and anchors



CHLORIDES
Expansion of
tendon or anchor,
creates spalls
& loss of section

CARBONATION
Lowers PH and
accelerates corrosion

CRACKS or LOW COVER
Allow corrosion
Promoters access
to tendon or anchor

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Strand Corrosion / Pitting Classifications

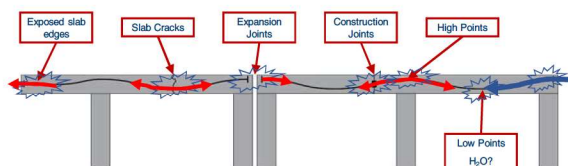


Loss of 3-4 wires under tension...
FAILURE!

PREV PROBLEMS NEXT

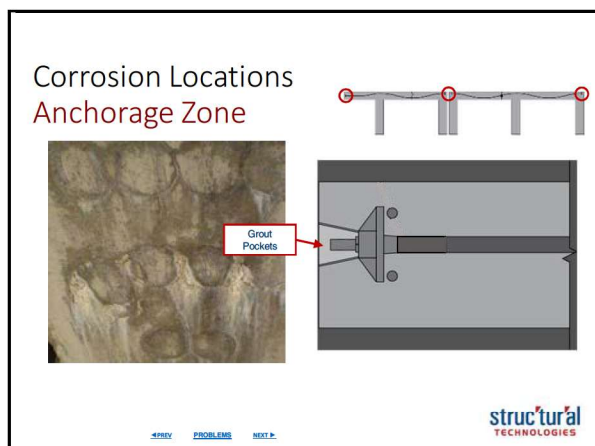
structural
TECHNOLOGIES

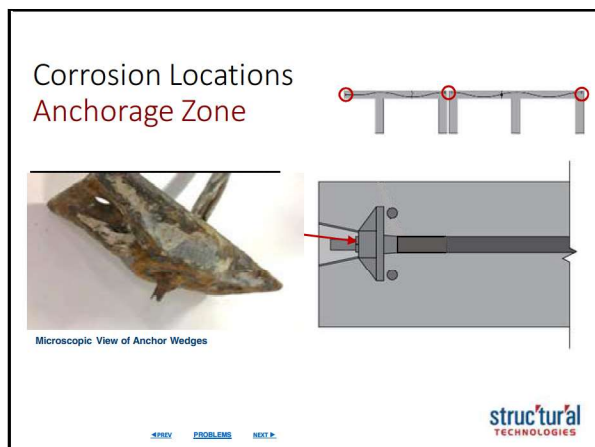
Typical Locations of PT Mild Steel Corrosion

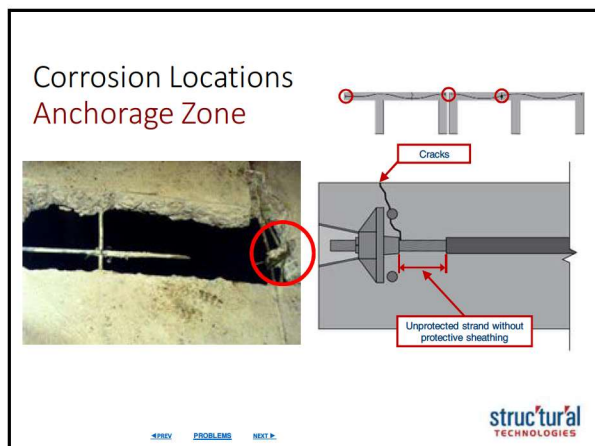


PREV PROBLEMS NEXT

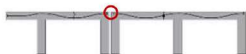
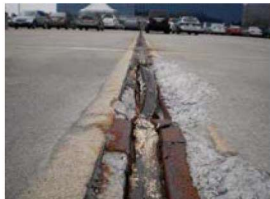
structural
TECHNOLOGIES







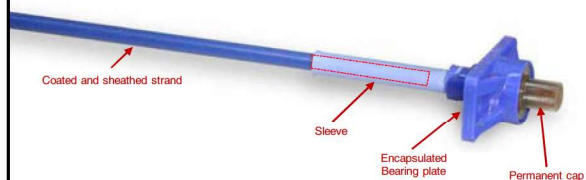
Corrosion Locations Expansion Joints



[« PREVIOUS](#) [PROBLEMS](#) [NEXT »](#)

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Modern Unbonded Monostrand Encapsulated PT Systems Eliminates water entry into tendon and anchorage

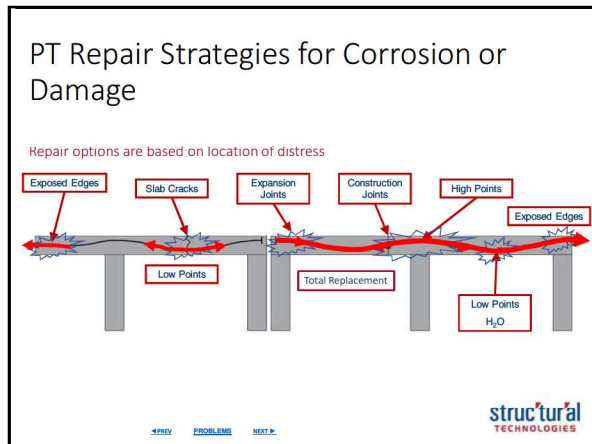


NOW REQUIRED BY CODE! Per ACI 318
(Except slab on grade)

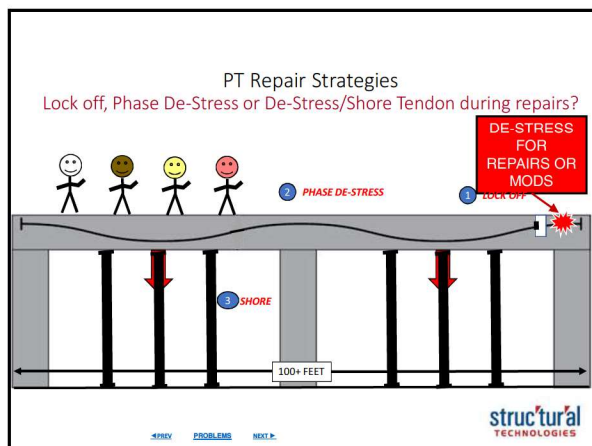
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TECHNOLOGIES

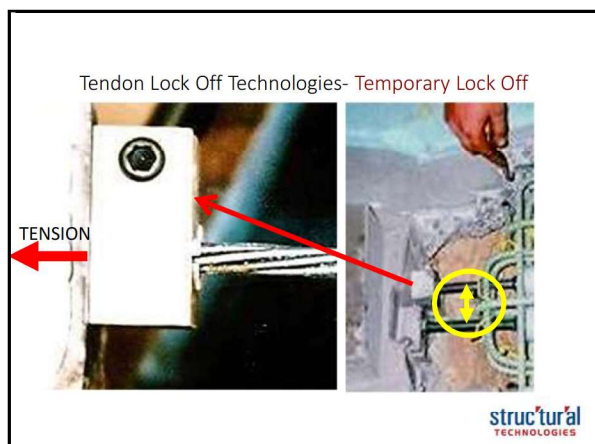
Repair Strategies for Solving Corrosion or Damage Problems with Post-Tensioned Concrete

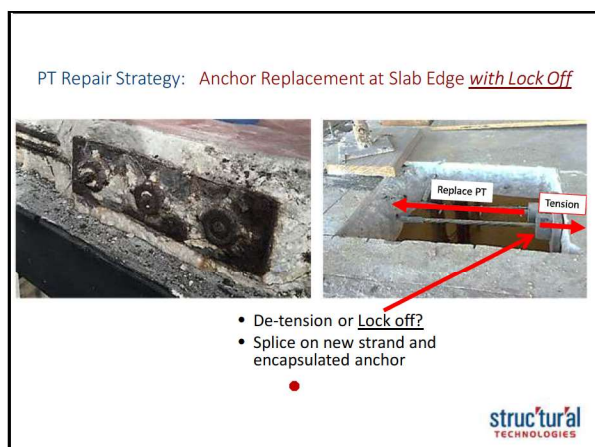
structural
TECHNOLOGIES

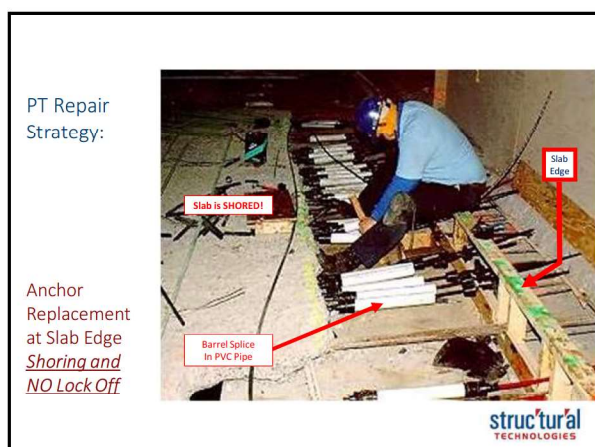


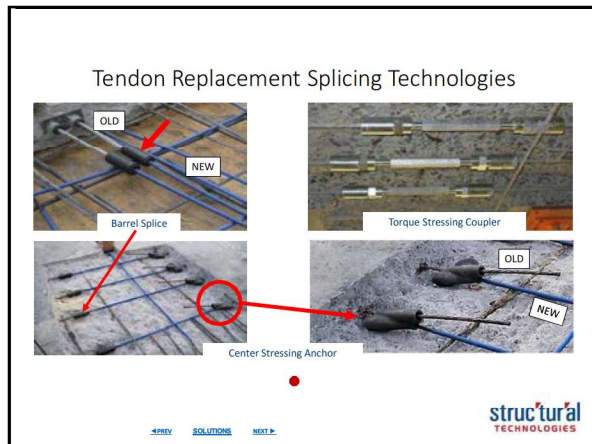


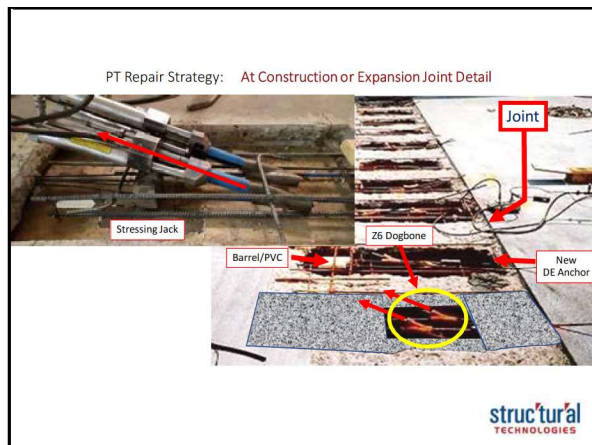


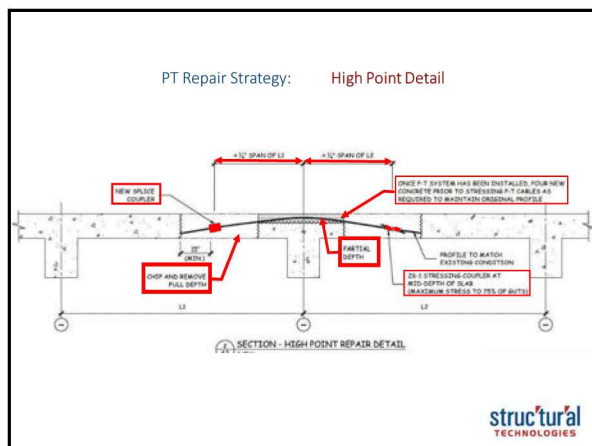


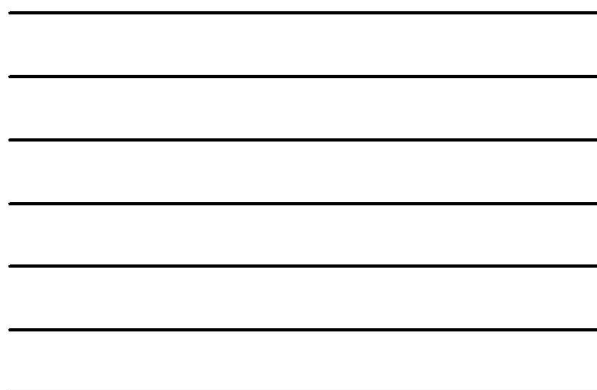


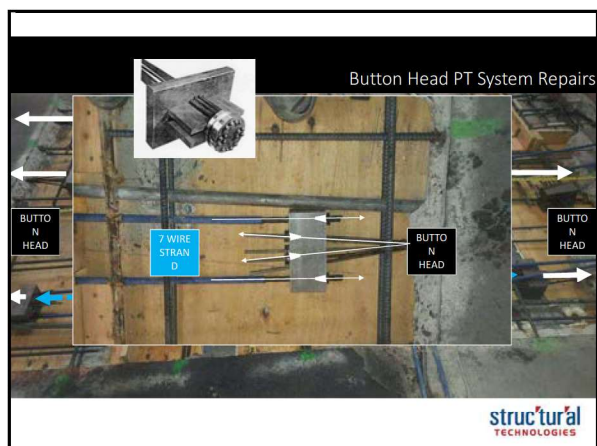


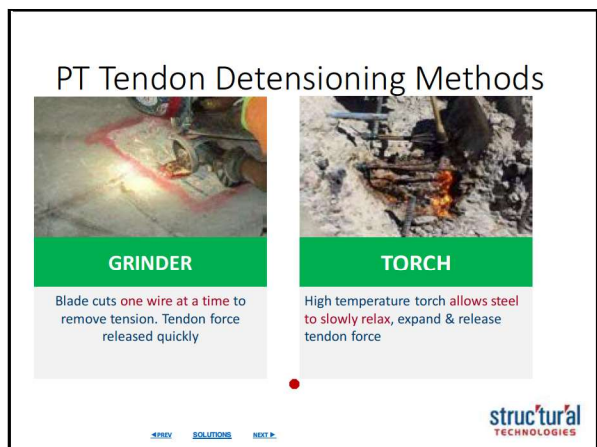


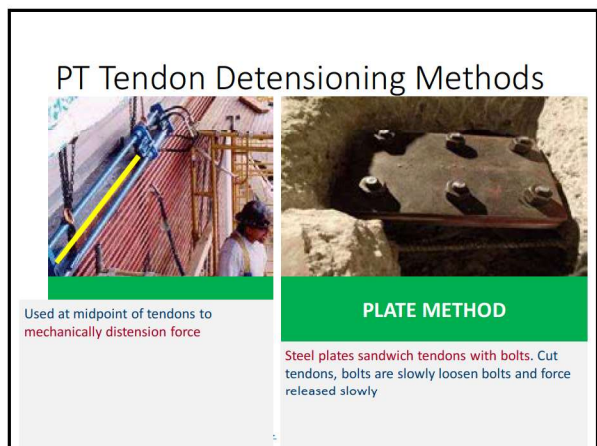


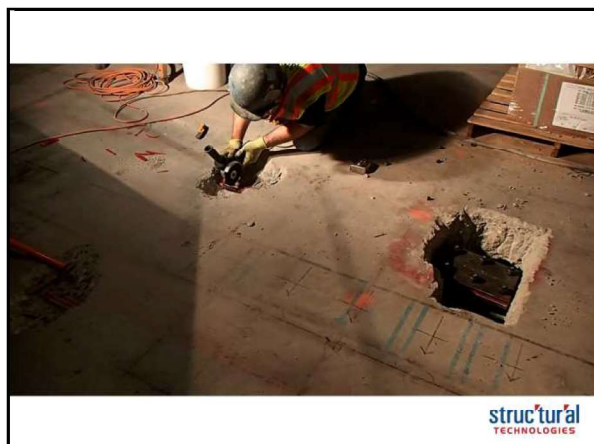


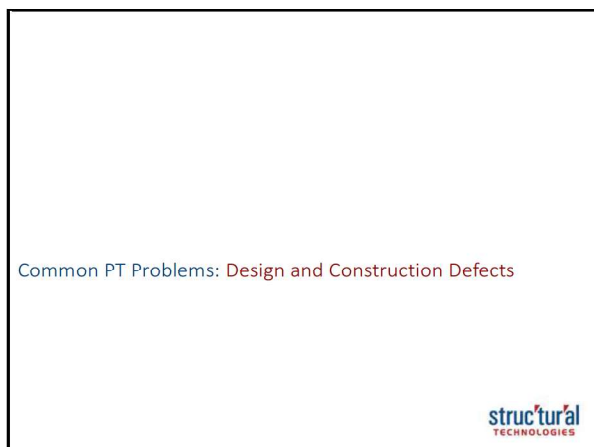


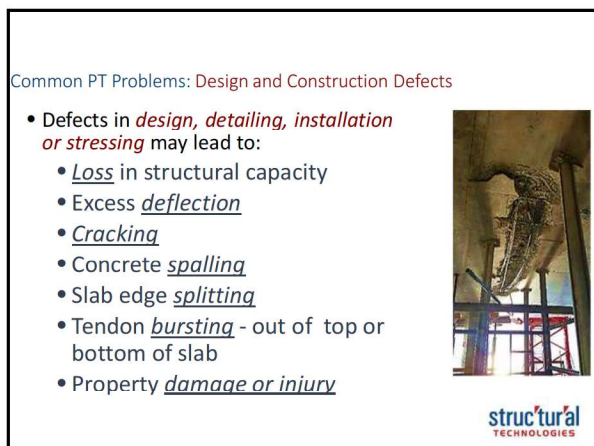


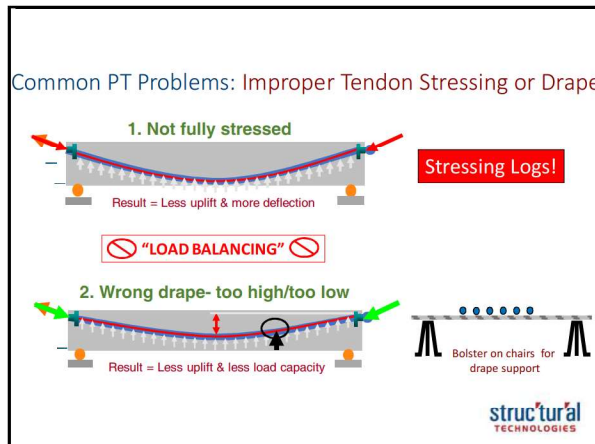




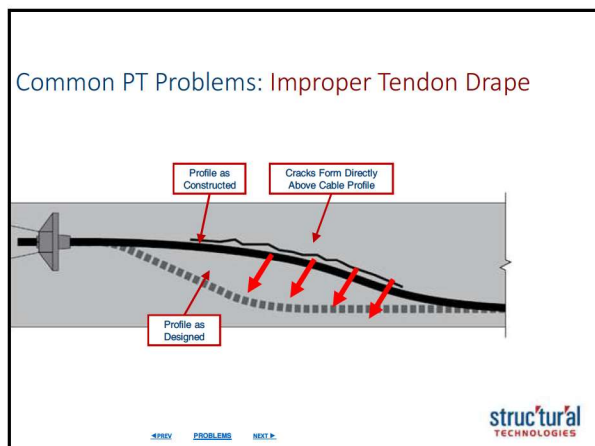




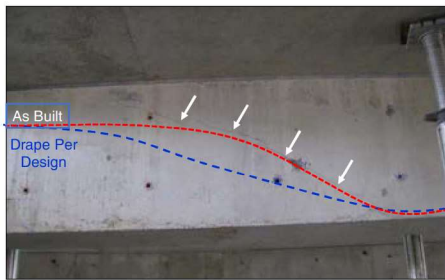








Common PT Problems: Improper Tendon Drape



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TECHNOLOGIES

Common PT Problems: Damaged Tendons



CORING/CHIPPING



SAW CUTTING



CONCRETE SCREWS

PREV PROBLEMS NEXT

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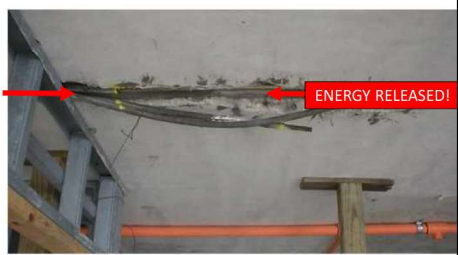
PT Blowout at High Point From corrosion, damage or anchor slip



PREV PROBLEMS NEXT

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PT Blowout at LOW Point From corrosion*, damage or anchor slip




*Broken corroded tendons do not always exit the concrete- WHY?

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◀ PREVIOUS PROBLEMS NEXT ▶

Common PT Problems Cracks from Overload or Loss of PT Force

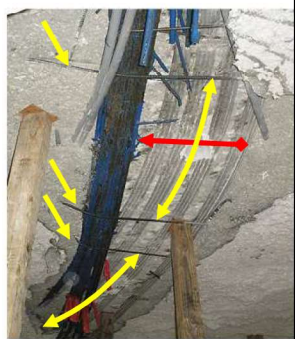


Higher sense of concern for structural cracking due to PT precompression forces

structural TECHNOLOGIES

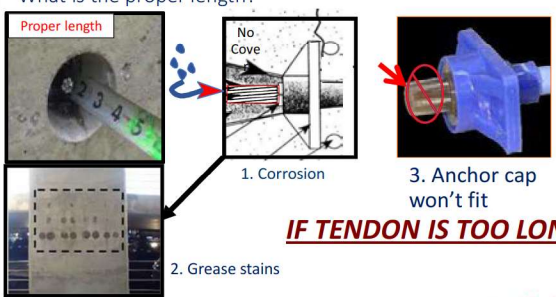
PT Sweep "Blowouts"

- Common Causes:
 - Inadequate "Hairpins" (confinement)
 - Sharp tendon sweeps



structural TECHNOLOGIES

Common PT Problems **Cutting Excess Tendon**
What is the proper length?



Proper length

No Cove

1. Corrosion

2. Grease stains

3. Anchor cap won't fit

IF TENDON IS TOO LONG

structural TECHNOLOGIES

Cutting Excess Tendon
What is the proper method

Grinder? Torch



Acetylene Torch OK?


OBSERVATION 5
Reference Photo

OBSERVATION 6
"WEDGES COMPROMISED BY HEATANCHOR CAN NOT BE ENCAPSULATED"

OBSERVATION 7
GREASE CAP INSTALLED BUT NOT SEALED

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Common PT Problems: **Cutting Excess Tendon AFTER** Stressing



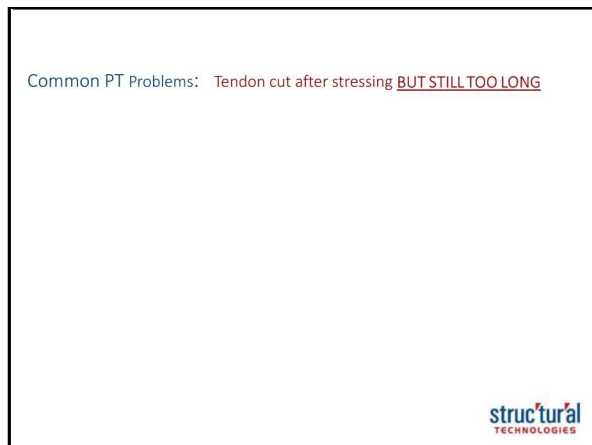
Hydraulic Cutter

Means and Methods OR Specification Guidance?

PER ACI 318 AFTER CUTTING
-Cap on in 6 hrs
Grout pocket in 1 day

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Large and Small New Openings in PT Slabs



New staircases
Mechanical openings

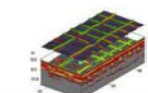


Elevator shafts/ducts



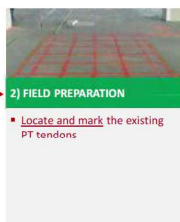
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TECHNOLOGIES

New PT Slab Opening Procedure (Overview)



1) ANALYSIS / DESIGN

- Analyze structural effect of slab opening
- Load changes in adjacent bays?
- If needed design proper shoring



2) FIELD PREPARATION

- Locate and mark the existing PT tendons



3) FIELD PT WORK

- Remove concrete over tendons at opening perimeter
- Lock off OR detension tendons?
- Demo opening
- Place new anchorages and reinforcement
- Place concrete
- Stress tendons

PREV PROBLEMS NEXT

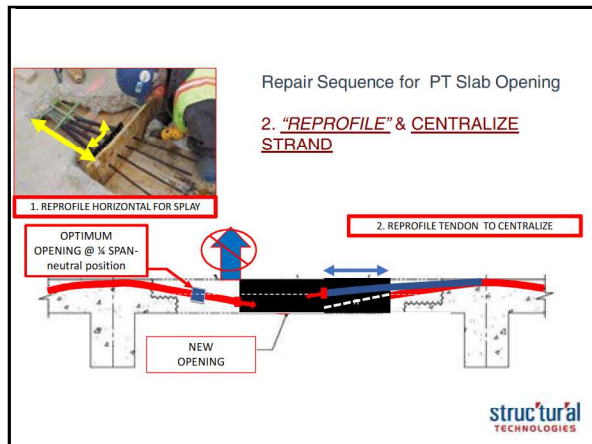
structural
TECHNOLOGIES

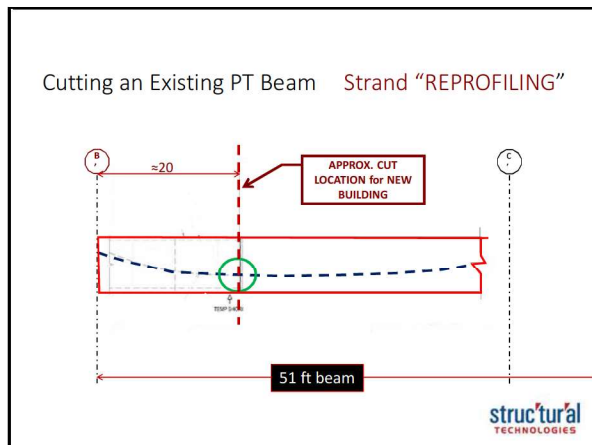
Repair Sequence for PT Slab Opening

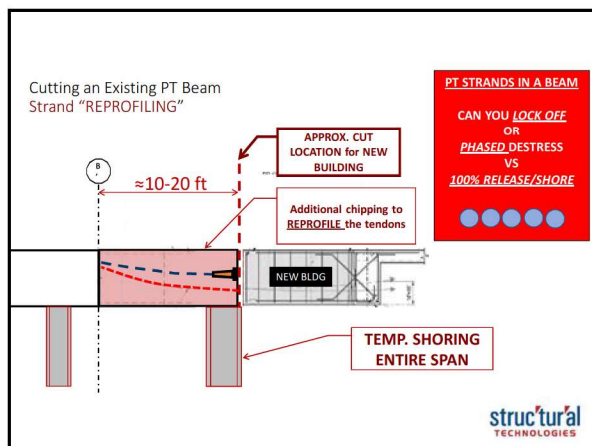
1. Create new anchor zone



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Completed Large Slab Openings



New atrium



New stairs

structural
TECHNOLOGIES

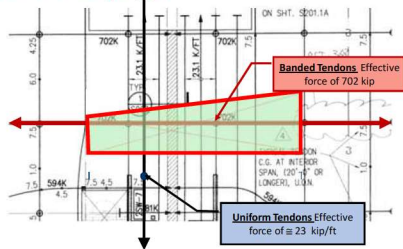
Case Study - Large Openings in PT Slab using Phased PT Release Approach



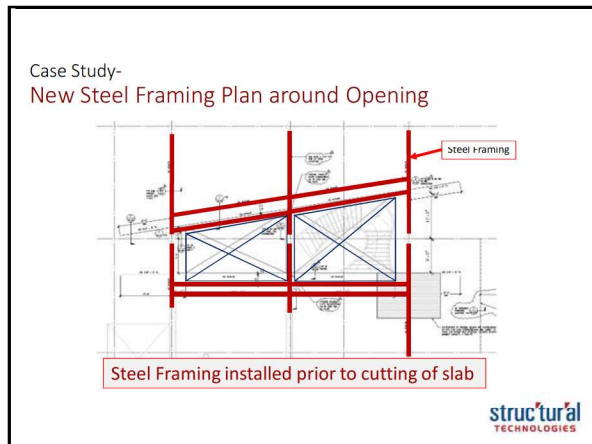
Large opening to accommodate new stairs

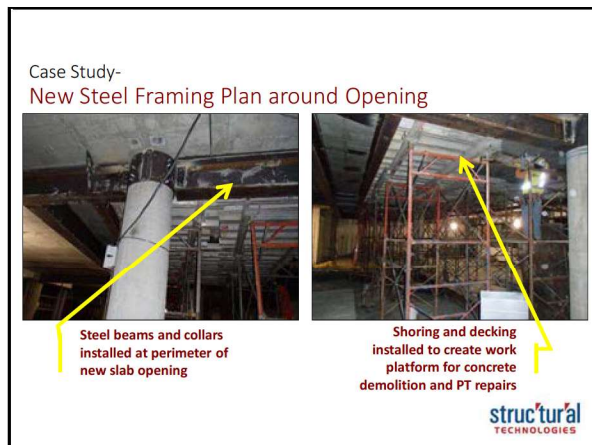
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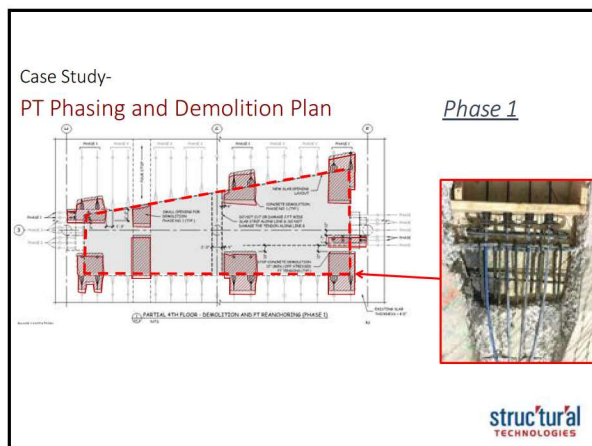
Case Study - Original PT Layout



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Case Study-

PT Phasing and Demolition Plan

Phase 2

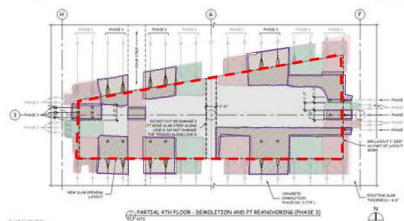


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Case Study-

PT Phasing and Demolition Plan —

Phase 3

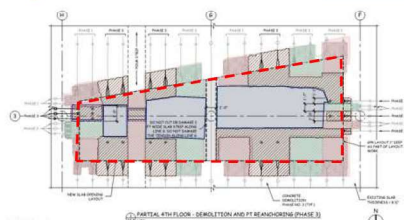


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Case Study-

PT Phasing and Demolition Plan

Final Phase 4

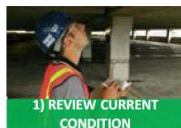


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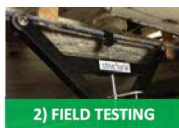
PT Structure Condition Assessment Process and Tools



PT Structure Condition Assessment



- Review PT **design and layout**
- Construction **sequencing**
- Review **exposure** conditions
- Visual inspection for **distress**



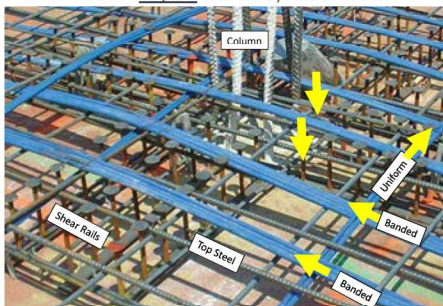
- Non-Destructive Testing (**NDT**)
- Exploratory Destructive Testing (**DT**)



- PT system material analysis
- Concrete testing



GPR or Other Before You CHIP, CORE or CUT!



Pachometer/GPR Survey

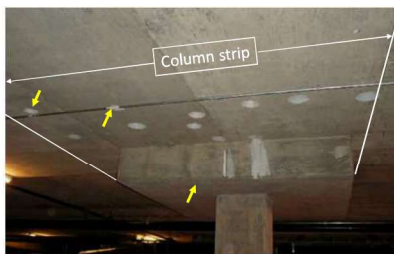
- Identify steel orientation and depth
- Compare findings to As Builts

NDT limitations – may be difficult
differentiating types of embedded
steel (PT vs rebar)



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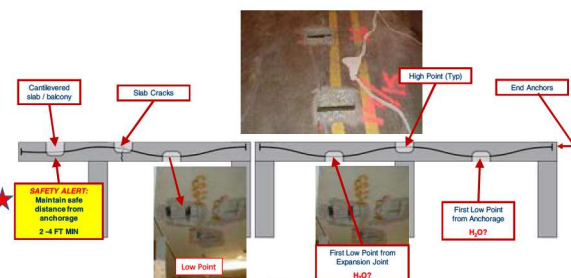
GPR or Other Before Coring!



Permission Not Forgiveness Rule!

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Typical High and Low Point Tendon and Anchor Inspection Points



• PREP • ASSESSMENT • NEXT •

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Checking Tendon at Low Point for Water



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Is There Tension in the Tendon? Screwdriver

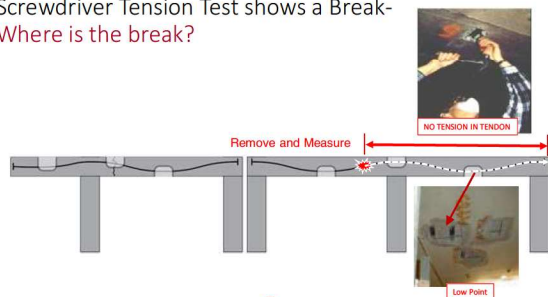
Tension Test



- Screwdriver driven between wires
- Used as a qualitative measurement if tendon is tensioned

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Screwdriver Tension Test shows a Break- Where is the break?



PREP ASSESSMENT NEXT

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In-situ Strand Tension Test – What is the in place tension?

- Applies load to a fixed length of tendon
- Measure effective force in tendon

Monostrand Anchor Insp

- Opening anchor pockets to:
 - Inspect wedges for corrosion & seating
 - Inspect anchor for corrosion
 - Anchor Cap inspection?

Half Cell Corrosion Potential Testing on Tendon at Anchor

- Estimates the electrical corrosion potential
- Gauges Probability of Active Corrosion

ASTM C876-90

Measured Potential* (mV/CSE)	Probability of Corrosion Activity
> -200	Less than 10%
-200 to -350	Uncertain
< -350	More than 90%


* CSE = Copper Sulfate Electrode

Laboratory Testing Techniques

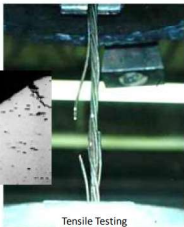
REVIEW TESTING ADVANCED TESTING

Testing of PT Components


- Corrosion/ pitting visual classification
- Hydrogen embrittlement of tendon?
- Analyze metal components with SEM microscopic investigation
- Grease / inspection testing
- Tensile testing of tendon




Anchor Teeth



Tensile Testing



PT Structure Safety Considerations



Post Tension SAFETY CONSIDERATIONS
During stressing or de-tensioning process, field investigation and repairs

STORED ENERGY IN A STRESSED TENDON



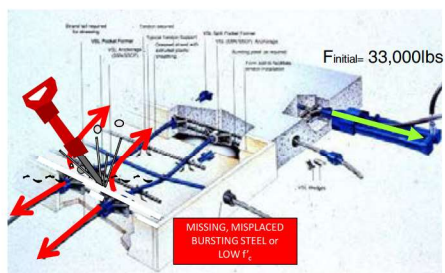
Common Causes:

- Inadequate/misplaced bursting steel
- Low strength concrete
- Concrete voids in anchorage zone
- Chipping behind the anchor



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During stressing or de-tensioning process, field investigation and repairs



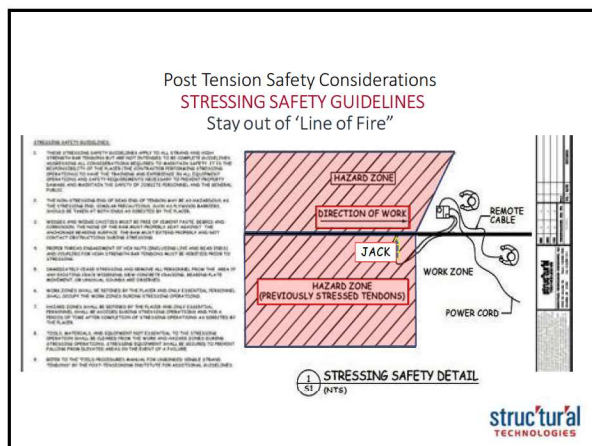
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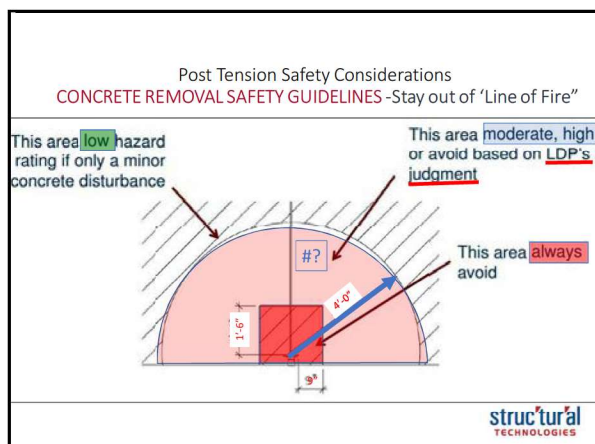
During stressing or de-tensioning process, field investigation and repairs

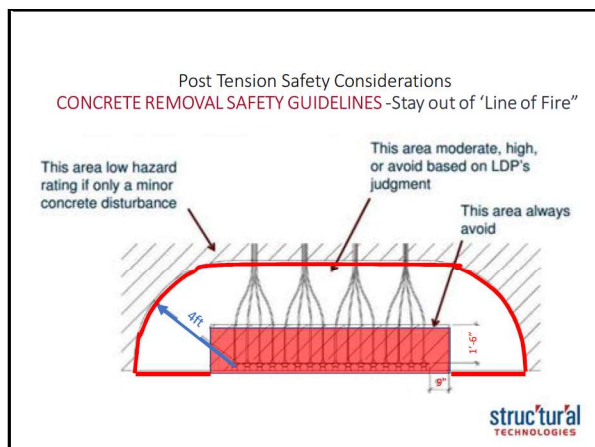


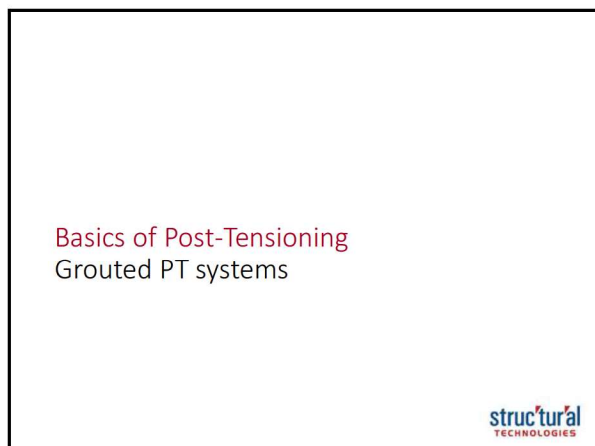


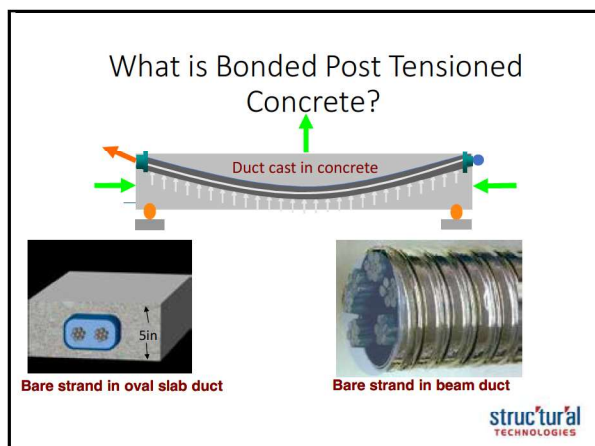


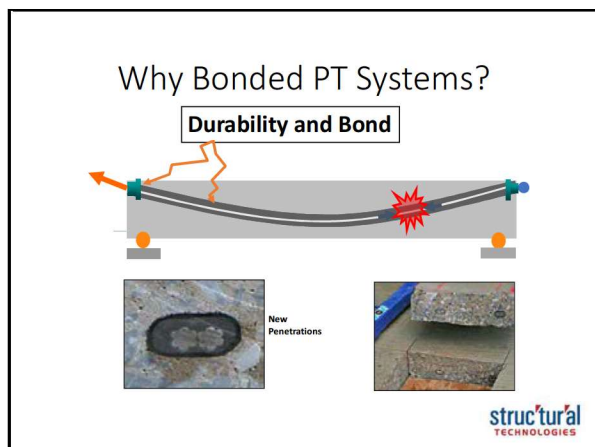


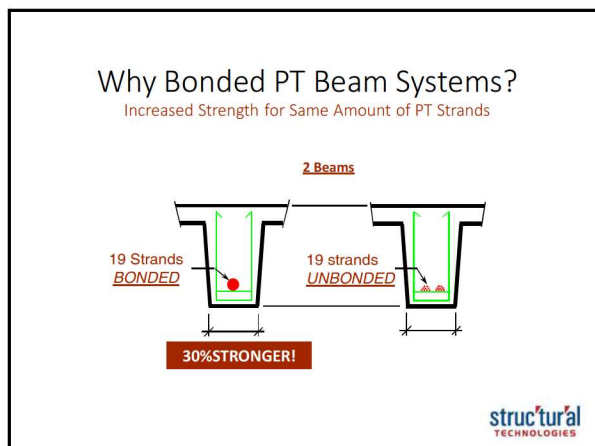








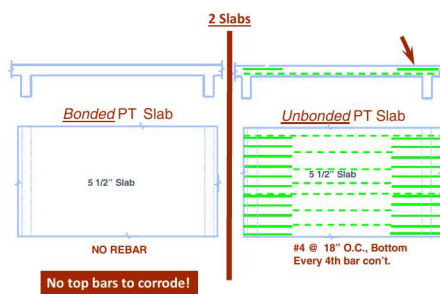




Why Bonded PT Slab Systems? Notice anything missing?



Why Bonded PT Systems? No top steel





Thank You!

Any Questions?

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443-271-7100